



# CBWC Series PRD3715

Form No: CBWC - 113

## Prepared For:

### PROVENT

3847 Wabash Dr. Mira Loma, CA 91725  
951.685.1101

## Prepared By:

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Job No.: PRO1103  
Effective Date: 2014.09.30



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For wood, concrete and steel attachments see Roof Anchorage Detail, Form No. CB-24A.

**STRUCTURALLY CALCULATED WELDED ROOF CURBS FOR YORK UNITS**

ProVent P/N	A	WEIGHT
CBWCPRD371508	8"	125 Lbs
CBWCPRD371511	11"	141 Lbs
CBWCPRD371514	14"	157 Lbs
CBWCPRD371524	24"	208 Lbs

**ZT, ZR, ZJ, ZH 037-150**  
**ZF, ZH, ZJ, ZR, XP, DH, DM, DF, DR, BP 078-150**

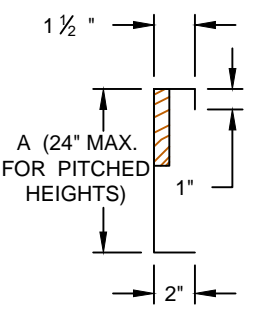
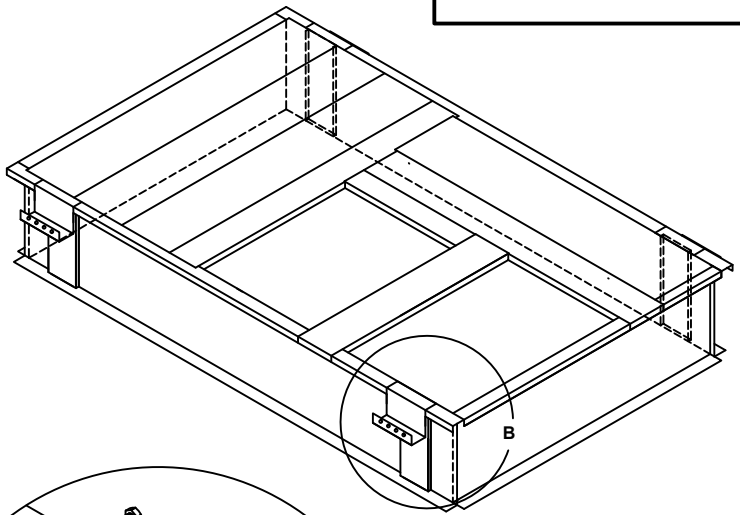
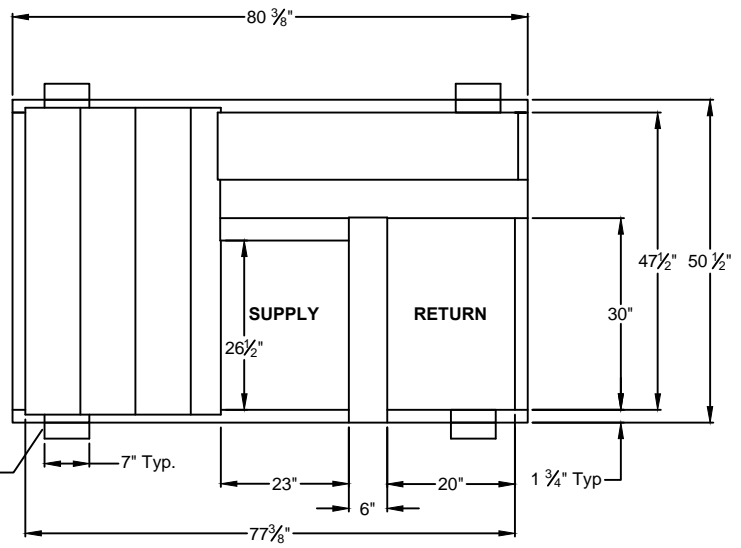
**Meets seismic requirements for the following codes:**  
 CBC 2013  
 IBC 2012

**FEATURES**

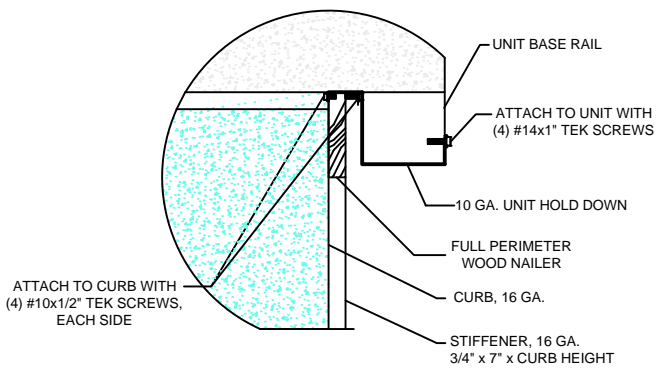
- Roof curb sides and ends are 16 ga. galvanized steel.
- Gasketing package provided.
- Heat treated wood nailer provided.
- Insulated deck pans provided.
- Pitched curbs and taller curbs are available.

**NOTES**

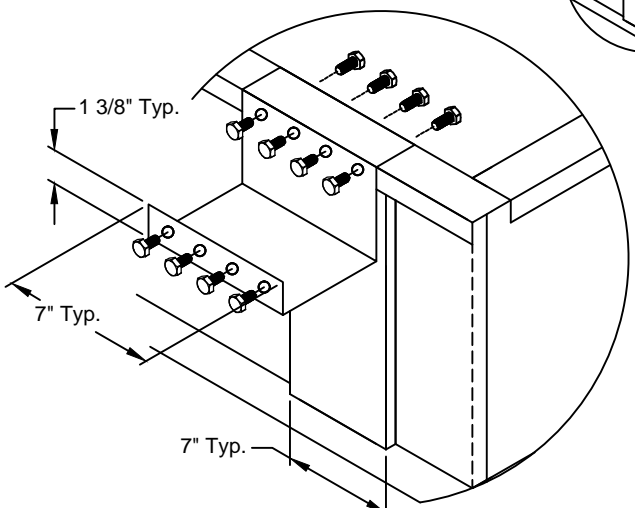
- Attach ductwork to roof curb. Flanges of duct rest on top of curb. Support ductwork below the curb.
- Thru the curbs utilities are available. Contact your York distributor or Provent directly.



**CURB DETAIL**



**HOLD DOWN DETAIL**



**DETAIL B**



3847 WABASH DR.  
 MIRA LOMA, CA 91725  
 PHONE (951) 685-1101  
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SUBMITTED TO: \_\_\_\_\_  
 COMPANY: \_\_\_\_\_  
 JOB NAME: \_\_\_\_\_  
 EQUIPMENT: \_\_\_\_\_  
 NOTES: \_\_\_\_\_

FORM NO: CBWC-113  
 DATE: 7/23/14  
 REV: 2

PART NUMBER: CBWCPRD3715 SERIES  
 DRAWN BY: JG

**Curb Information**

HVAC UNIT: Predator Welded Roof Curb 8-14"

Curb Number:

Hcurb	24	(Height from support structure to top of curb)
Lcurb	77.375	(Length of Curb - In to In)
Wcurb	47.5	(Width of Curb - In to In)
Lclip	66.625	(min Length in Long dir from end to clip)
# clips Long Side	2	(Shear + Uplift Clips)
# clips short side	0	(Shear Clips)

**Unit Information:**

Weight:	2095 lbs	(Max Weight)
W c-max	719 lbs	(Maximum corner weight)
W c-min	293 lbs	(Minimum corner weight)
W mid	300 lbs	
H unit	50.75	(Height of unit above curb)
H cm	25.38	(Height from top of curb to center of mass of unit)
L unit	120.00	(Length of unit)
w unit	59.00	(Width of unit)

**Seismic Loading**

	CBC2012	(DESIGN ALSO OK FOR IBC 2009/CBC2010)
Ss	2.00	
Fa	1.00	(worst case for site; Ss>1.25, Site Class D)
Sms	2.00	(=Fa*Ss)
Sds	1.33	(=2/3*Sms)
Ip	1.5	(=Worst case)
ap	2.5	
Rp	6	
Fp max	1.0 Wp	(=0.4*ap*Sds*Wp*(1+2*z/h)/(Rp/Ip))
Fp ASD	1534 lbs	(=0.7*Fpmax)

**Wind Loading (Ultimate)**

Code:	CBC2012	(DESIGN ALSO OK FOR IBC 2009/CBC2010 - 90MPH)
Wind Exposure:	C	
V	120 mph	(Ultimate Wind Speed)
Kzt	1	
Kd	0.9	
Kh	1.13	for 60 foot roof height
GCf	1.9	

**Wind Loading: Lateral**

qh	37 psf		(ASD=0.6W)
F	71 psf	(ultimate)	43 psf (service)
A net/transv.	42 sf		
Fwind transv	3013 lbs	(ultimate)	Fwind trnsv 1808 lbs (service)
A net/long	21 sf		
Fwind long	1481 lbs	(ultimate)	Fwind long 889 psf (service)

**Controlling Lateral Load (Seismic vs. Wind)**

Transverse	1808 lbs (ASD)
Long	1534 lbs (ASD)

**Wind Loading: Uplift**

qh	37.5 psf		
GCr	1.5		
Fv (psf)	56.2 psf	(ultimate)	
Av	49 sf		
Fv	2765 lbs	(ultimate)	

Fv	1659 lbs	(ASD: 0.6W)
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**Curb Loading:**

Transverse direction:

OTM:	3823 lb-ft	0.6W: F max - transv * Hcm
M res.	1516 lb-ft	0.6D: 2 * Wcrnmin * 0.6* (width curb +4.25")
Net OTM:	2307 lb-ft	0.6D+0.6W: OTM-RM
Max Comp	1738 lbs /side	1.0D+0.6W: (2 x Wcrnmax + (Mot-Mres)/ (width curb +4.25"))
Max Tension	1364 lbs /side	0.6D+0.6W: (Mot-Mres)/(width curb +4.25") + Fv/(2 sides)
Max Tension	682 lbs /clip	0.6D+0.6W: Max tension/side /#clips
Max Corner:	885 lbs	W c-max+ (Mot-Mres)/((w/curb+4.25")/Lcurb*(N-corner)/2/12
Max Interior:	632 lbs	W mid+ (Mot-Mres)/(width curb +4.25")/Lcurb*(N-mid)

Longitudinal direction:

OTM:	3243 lb-ft	0.6W: F max long * Hcm
RM	2399 lb-ft	0.6D: 2 X Wcrnmin x (Lcurb+4.25in)/12 x 0.6
Net OTM:	845 lb-ft	0.6D+0.6W: OTM-RM
Max Comp =	1302 lbs /side	1.0D+0.6W: (2*Wcmax + (NetOTM)/Lclip)+ Fv/(U.C)
Max Tens =	982 lbs /side	0.6D+0.6W: (Mot-Mres)/Lclip + Fv/(2 sides)
Max Tension =	567 lbs /clip	0.6D+0.6W: (Mot-Mres)/Lclip + Fv/(2 sides)/#clips/side
Max Corner:	785 lbs	W c-max+ (Mot-Mres)/Lcurb/Wcurb*(N-corner)/2

**Connection of Unit to Curb**

Screws - metal ga

Screws in Uplift clips	#10-16ga	Vall=	582	lbs/screw
Screws in Short Side Clips	#10-16ga	Vall=	582	lbs/screw

UPLIFT CLIPS:

Shear:	383 lbs/clip
Uplift:	682 lbs/clip
F diag	513 lbs/clip to curb

UPLIFT CLIPS:

	2	#10-16ga
Fdiag/screw		257 lbs/screw
F/screw outs.		171 lbs/screw

SCREWS OK

SHORTSIDE CLIPS

Shear:	#DIV/0!
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SHORTSIDE CLIPS

	2	#10-16ga
F screw		#DIV/0! lbs/screw
		#DIV/0!

**Curb info**

H curb	24	in, curb height max
Fy	50	ksi
E	29000	ksi
Fu	65	ksi
gage:	16	
R	0.0849	
t	0.0593	
R/t	1.50	
h	23.77	in

**Interior Curb**

N	48	bearing length: 2*h
C	20	
C <sub>R</sub>	0.1	
C <sub>N</sub>	0.08	
C <sub>h</sub>	0.03	
φ <sub>w</sub>	0.85	
Ω <sub>w</sub>	1.75	
P <sub>n</sub>	4.04 k	
P <sub>all</sub>	2.31 k	
φP <sub>n</sub>	3.43 k	
P <sub>max</sub>	0.63 k	P <sub>all</sub> >P <sub>max</sub> ,curb OK,

**Exterior Curb:**

N total	48	2*curb height
N -BUS	7	bearing length with stiffener (built up section)
N - SWC	41	bearing length without stiffener (single web channel)

Exterior Curb SWC	
N	41
C	7.5
C <sub>R</sub>	0.08
C <sub>N</sub>	0.12
C <sub>h</sub>	0.05
φ <sub>w</sub>	0.85
Ω <sub>w</sub>	1.75
P <sub>n</sub>	-0.01 k
P <sub>all</sub>	0.00 k
φP <sub>n</sub>	0.00 k

Exterior Curb BUS	
N	7
C	15.5
C <sub>R</sub>	0.09
C <sub>N</sub>	0.08
C <sub>h</sub>	0.04
φ <sub>w</sub>	0.75
Ω <sub>w</sub>	2
P <sub>n</sub>	1.81 k
P <sub>all</sub>	0.90 k
φP <sub>n</sub>	1.35 k

Corner:	
P <sub>all</sub>	0.90 k
φP <sub>n</sub>	1.35 k
P <sub>max</sub>	0.88 k

P<sub>all</sub>>P<sub>max</sub>,curb OK,

**Connection of Curb to Supporting Structure (ASD)**Transverse: **0.6D+0.6W**

\*\*Uplift (wind)= 2173 lbs  
 \*Uplift (seismic) 1105 lbs  
 Uplift max= 2173 lbs/long side  
 \*\*Shear (wind) 2359 lbs  
 \*Shear (seis) 1534 lbs  
 Shear max= 2359 lbs/total curb

Longitudinal:

\*\*Uplift (wind)= 1086 lbs  
 \*Uplift (seismic) 705 lbs  
 Uplift max= 1086 lbs/long side  
 \*\*Shear (wind) 1227 lbs  
 \*Shear (seis) 1534 lbs  
 Shear max= 1534 lbs/total curb

T max 

2173
------

 lbs/long  
 V max 

2359
------

 lbs/total

**Wood Attachment: (Use 1/4"  $\phi$  x 3.5" SDS SCREWS)  
(3.5" minimum embed into DF or SP wood)**

W' 

616
-----

 lbs  
 V' 

256
-----

 lbs

total screws required= 

10
----

  
 # screws: LONG SIDE 

4
---

  
 # screws: SHORT SIDE 

1
---

  
 W/screw (uplift) 2 lbs  
 V/Screw 236 lbs

WOOD SCREWS OK**Steel Deck Attach.: (Use 5/8"  $\phi$  A307 Bolts attached to L5x5x1/4 below deck at each conn.point)**

Tall= 

6900
------

 lbs  
 Vall= 

3680
------

 lbs

total bolts required= 

6
---

  
 # bolts: LONG SIDE 

2
---

  
 # bolts: SHORT SIDE 

1
---

  
 T/bolt (uplift) 1087 lbs  
 V/bolt 393 lbs  
 fv 1.3 ksi  
 ft 3.5 ksi  
 Fnt' 53.7 ksi  
 Fnv 24.0 ksi

OK  
OK

BOLTS OK

**Connection of Curb to Concrete Supporting Structure - STRENGTH DESIGN**

Transverse: **0.9D +1.0(E or W)**

**Uplift (wind)=	3681 lbs	
*Uplift (seismic)	10291 lbs	*Rp*1.3/1.5/0.7asd
Uplift max=	10291 lbs/side	(Per long side curb)
**Shear (wind)	3931 lbs	(Maximum Lateral Force) (seismic: Rp=1.5 max)
*Shear (seis)	11392 lbs	*Rp*1.3/1.5/0.7asd
Shear max=	11392 lbs	(Total Curb)

Longitudinal:

**Uplift (wind)=	1882 lbs			
*Uplift (seismic)	7795 lbs		Tu max <table border="1" style="display: inline-table;"><tr><td>10291</td></tr></table> lbs	10291
10291				
Uplift max=	7795 lbs/side	(Per long side curb)	Vu max <table border="1" style="display: inline-table;"><tr><td>11392</td></tr></table> lbs	11392
11392				
**Shear (wind)	2045 lbs			
*Shear (seis)	11392 lbs			
Shear max=	11392 lbs	(Total Curb)		

**Concrete Attach.: (Use 5/8" ϕ Simpson Strong Bolt 2)**

phi-Tn	2600	lbs
phi-Vn	1100	lbs

(LRFD design)

Anchors Required for Uplift (long side only)=	4.0
Anchors Required for Shear (Total Curb)=	10.4
Anchors Required (long side only) =	4.0

**Conc Anchors**

anchors (long side)	4.0	OK
anchors (short side)	2.0	
anchors (total)	12	OK
Tu	2573	OK
Vu	949	OK

Simpson Strong Bolt 2  
 3 5/8" Embed  
 (f'c=4000psi, 6" min total thickness - normal weight concrete, 12" E.D.)  
 ESR Report - 3037  
 Special Inspection Required

\*Uplift and Shear seismic anchorage forces have been designed for an Rp of 1.5 max per ASCE 13.4.2

Anchorage design per ASCE 14.2.2.17/ACI D3.3 with strength reduction factors not required in combination with Rp=1.5

\*\* For wind force, shear at base for anchorage design, accounts for add'l area from curb width and height