MOUR GROUP
ENGINEERING + DESIGN

6593 Riverdale St.
San Diego, CA 92120
619-727-4800

Structural Calculations
for
CBISC-01 Series

CBISCLXS** SERIES

Prepared for:

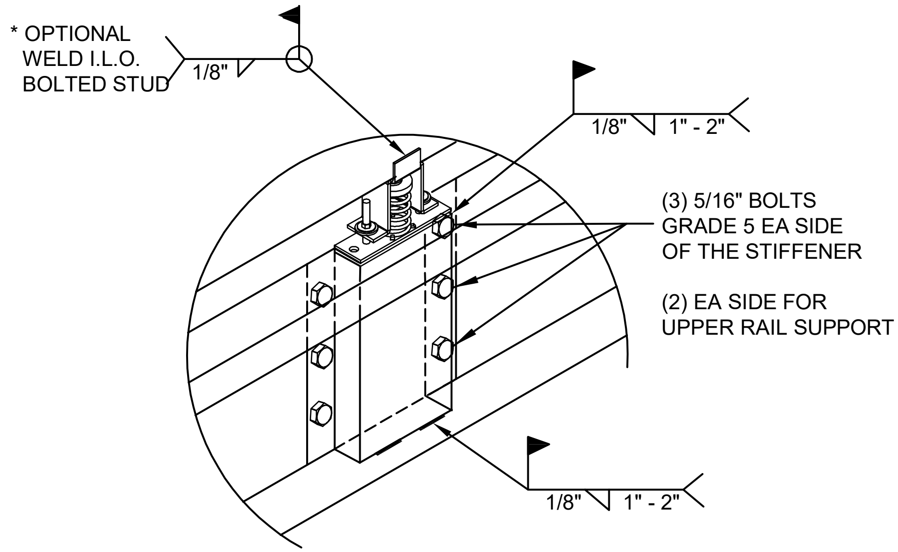
PROVENT / RRS

3847 Wabash Drive
Mira Loma, CA 91725

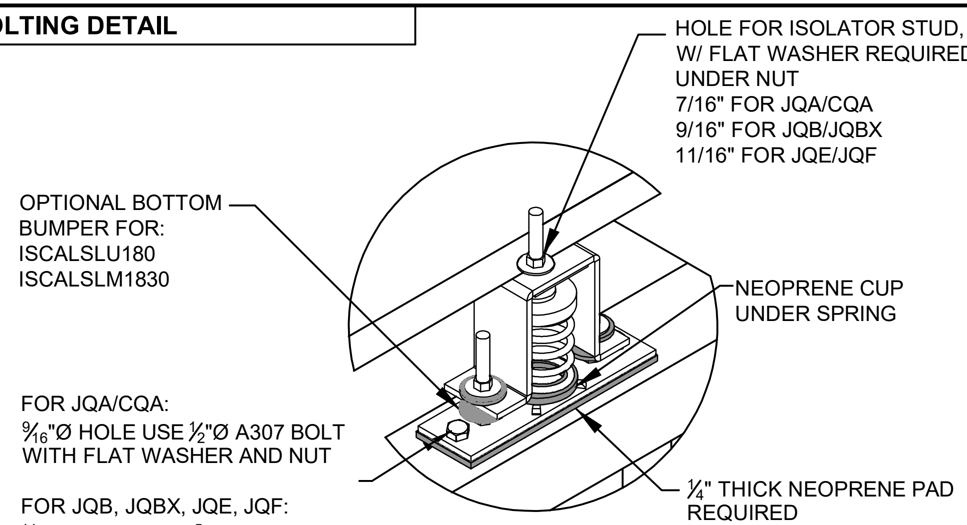
Date: August 23, 2023

Project Number: PV2312

WELDMENT AND BOLTING DETAIL



BASE CURB SUPPORT



FOR BOLT ON ISOLATORS

Note: * - INDICATES WELD REQUIRED I.L.O.
BOLTED STUD FOR THE FOLLOWING CURBS:

- -LXL (CBISC-02)
- -PRD3715 (CBISC-04)
- -SAV1518 (CBISC-12)
- -SAV2025 (CBISC-13)
- -SAV28 (CBISC-14)

ProVent

3847 WABASH DRIVE
MIRA LOMA, CA 91725

PHONE (951) 685-1101
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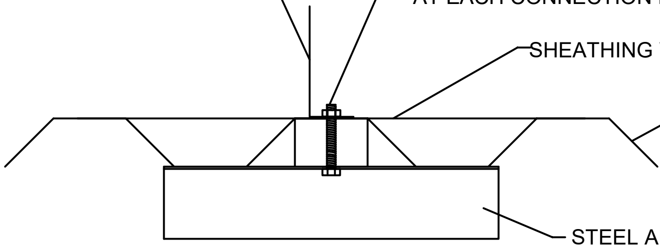
SUBMITTED TO:__________
COMPANY:__________
JOB NAME:__________
EQUIPMENT:__________
NOTES:__________

FORM NO:		
CB-61		
DATE:	REV:	DRAWN BY:
08/14/23	2	FMM

STEEL ATTACHMENT



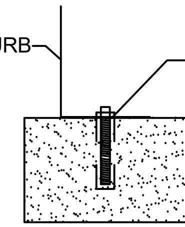
CURB	NO. OF ANCHORAGE BOLTS REQUIRED	
	LONG SIDE	SHORT SIDE
LXS	3 @ 19.25" O.C.	2 @ 23" O.C.
LXL	3 @ 19.25" O.C.	2 @ 33" O.C.
SUN3672	4 @ 21" O.C.	2 @ 27.25" O.C.
PRD3715	6 @ 14.28" O.C.	3 @ 20.75" O.C.
PRS	4 @ 20.46" O.C.	2 @ 31.13" O.C.
PRL	5 @ 17.44" O.C.	2 @ 41.5" O.C.
SAV1518	6 @ 22.43" O.C	3 @ 35.56" O.C.
SAV2025	7 @ 21.02" O.C	3 @ 35.56" O.C.
SAV28	7 @ 23.75" O.C	3 @ 35.56" O.C.

ASSUMES:
CONC SLAB
f'c= 4000PSI MINIMUM
6" MIN THICKNESS
NORMAL WEIGHT CONCRETE
MIN. 9-1/8" EDGE DISTANCE.

Meets seismic requirements for the following codes: CBC 2022 IBC 2021

ROOF ANCHORAGE DETAIL
CBISC Series
LXS
LXL
SUN3672
PRD3715
PRS
PRL
SAV1518
SAV2025
SAV28

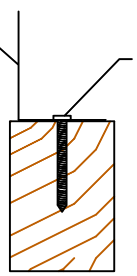
CONCRETE ATTACHMENT



CURB	NO. OF ANCHORAGE BOLTS REQUIRED	
	LONG SIDE	SHORT SIDE
LXS	4 @ 12.83" O.C.	2 @ 23.0" O.C.
LXL	4 @ 12.83" O.C.	3 @ 16.50" O.C.
SUN3672	4 @ 21.0" O.C.	2 @ 27.25" O.C.
PRD3715	9 @ 8.92" O.C.	6 @ 8.30" O.C.
PRS	5 @ 15.34" O.C.	3 @ 15.56" O.C.
PRL	7 @ 11.63" O.C.	4 @ 13.83" O.C.
SAV1518	8 @ 16.02" O.C.	6 @ 14.23" O.C.
SAV2025	9 @ 15.77" O.C.	6 @ 14.23" O.C.
SAV28	10 @ 15.83" O.C.	6 @ 14.23" O.C.

*** SIX INCHES FROM EACH CORNER EVENLY SPACED.**
**** CENTERED.**

WOOD ATTACHMENT



FOUR INCHES FROM EACH CORNER EVENLY SPACED

CURB	NO. OF ANCHORAGE SCREWS REQUIRED	
	LONG SIDE	SHORT SIDE
LXS	8 @ 6.07" O.C.	5 @ 6.75" O.C.
LXL	7 @ 7.08" O.C.	7 @ 6.17" O.C.
SUN3672	9 @ 8.38" O.C.	5 @ 7.81" O.C.
PRD3715	15 @ 5.38" O.C.	10 @ 5.06" O.C.
PRS	10 @ 7.26" O.C.	6 @ 7.03" O.C.
PRL	12 @ 6.70" O.C.	8 @ 6.50" O.C.
SAV1518	15 @ 8.29" O.C.	10 @ 8.35" O.C.
SAV2025	18 @ 7.65" O.C.	10 @ 8.35" O.C.
SAV28	20 @ 7.71" O.C.	10 @ 8.35" O.C.

ProVent
3847 WABASH DRIVE
MIRA LOMA, CA 91752
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SUBMITTED TO: ____________
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JOB NAME: ____________
EQUIPMENT: ____________
NOTES: ____________

FORM NO:		
CB-62		
DATE:	REV:	DRAWN BY:
6/28/2023	4	FMM

For wood, concrete and steel attachment see Roof Anchorage Detail, Form No. CB-62.

Welded Isolation springs housingare standard. For bolted spring housing, neoprene pads and spring cups see Weldment and Bolting Detail, Form No. CB-61

STRUCTURALLY CALCULATED VIBRATION ISOLATION ROOF CURB FOR LX SERIES SMALL CHASSIS UNITS

P***A CABINET

PROVENT P/N	A	B	EST. WEIGHT
CBISCLXS18*	8"	18"	180 Lbs.
CBISCLXS21**	11"	21"	195 Lbs.
CBISCLXS24**	14"	24"	210 Lbs.

**Note: Spring configuration must be added to part number at time of order

Weight of upper portion supported by spring isolators= 111 Lbs.

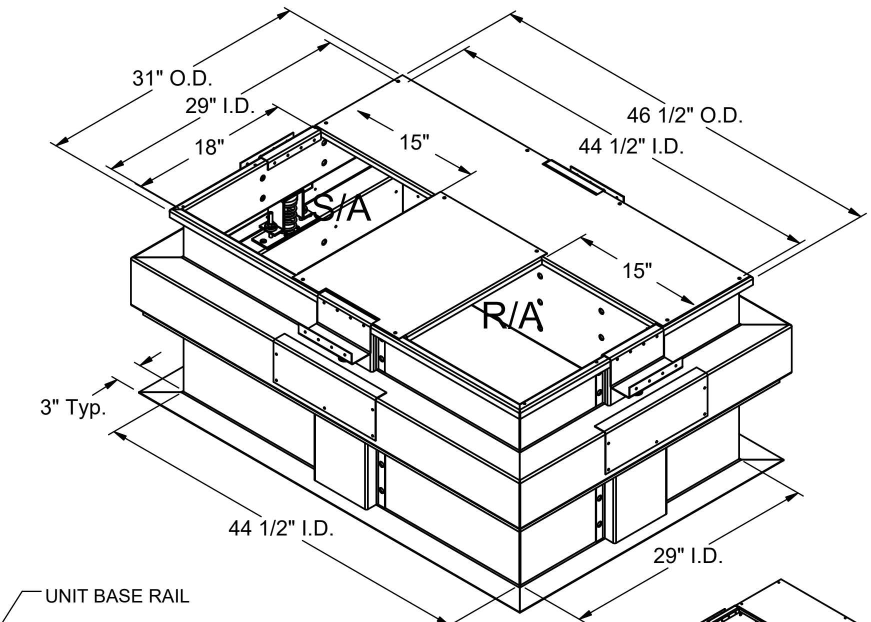
Meets seismic requirements for the following codes:
CBC 2022
IBC 2021

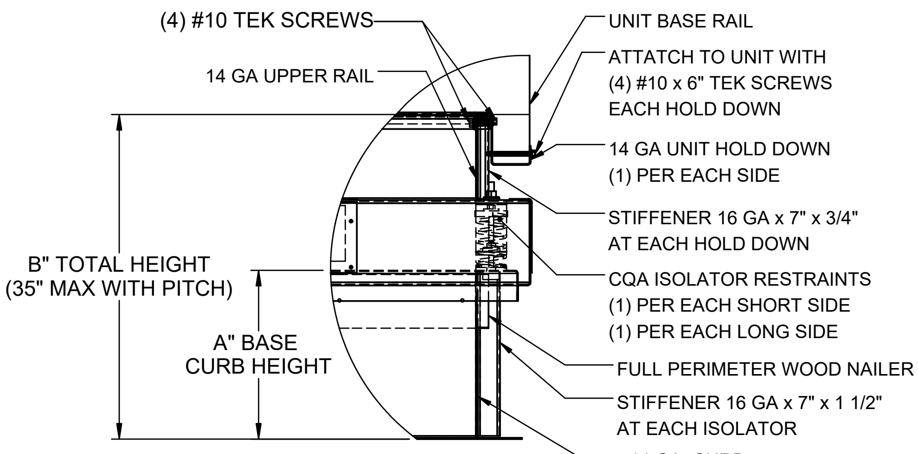
FEATURES

- Roof curbs sides and ends are 14 ga.
- Fully welded construction.
- Gasketing package provided.
- Heat treated wood nailer provided.
- insulated deck pans provided.
- Pitched curbs and taller curbs are available.
- CalDyn OSHPd pre-approved seismic restraints. (OPM-0401-13), (CQA).

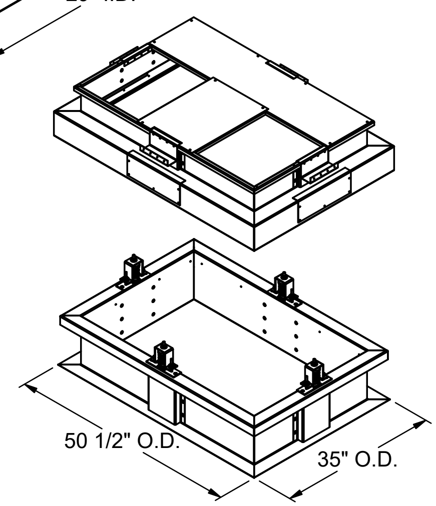
NOTES

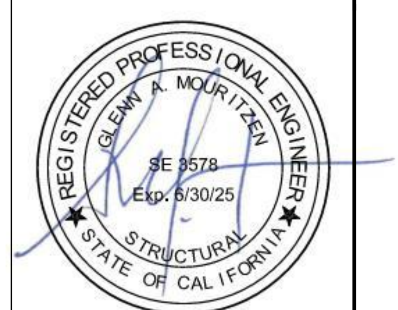
- Attach ductwork to roof curb. Flanges of duct rest on top of the curb, Support ductwork below the curb.
- Thru the curb utillities are available. Contact you York distributor or Provent directly.





PROFILE DETAIL





ProVent

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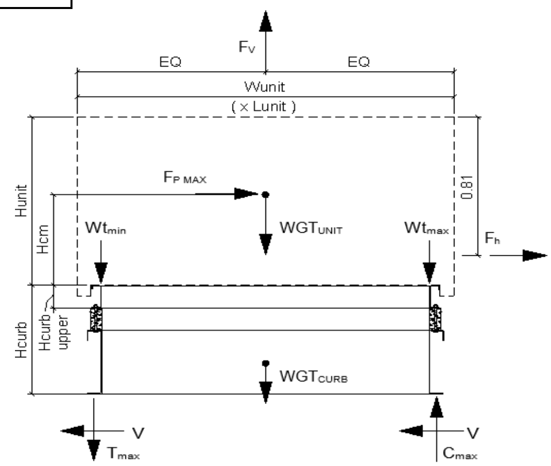
SUBMITED TO: ____________
COMPANY: ____________
JOB NAME: ____________
EQUIPMENT: ____________
NOTES: ____________

FORM NO:	PART NUMBER:	
CBISC-01	-	
DATE:	REV:	DRAWN BY:
7/28/2023	5	JG

6593 Riverdale St.
San Diego, CA 92120
(619)727-4800

Client:	ProVent	PV2312	Upper curb rail
Project:	CBISC-01	Iso Curb	CBISCLXS
Unit:	ALL P***A CABINET		



Upper Curb Information

Hcurb upper =	5.5	in	(Height of upper curb rail)
Lcurb =	46.5	in	(Length of upper curb)
wcurb =	31	in	(Width of upper curb)
WGTupper =	111	lbs	(Weight of upper curb)
# Clips long side =	1		# Clips short side = 1

Unit Information

WGTunit =	521	lbs	(Weight of Unit)
Wtmax =	156	lbs	(Maximum corner weight)
Wtmin =	111	lbs	(Minimum corner weight)
Hunit =	49	in	(Height of unit above curb)
Hcm =	24.5	in	(Height to center of mass)
Lunit =	51.25	in	(Length of unit)
Wunit =	35.75	in	(Width of unit)

Seismic Loading - 2021 IBC/2022CBC

Ss =	2.85		(Worst case for majority of California)		
Fa =	1.20		(Default Site Class D - Table 11.4-1 ASCE 7-16)		
Ip =	1.50		(Importance Factor Category III Building)		
Sms =	3.420		(Fa*Ss)	ap =	2.5
Sds =	2.280		(2/3*Sms)	Rp =	2
Fpmax =	5.130	Wp	(0.4*ap*Sds*Ip)*Wp*3/Rp <=1.6*Sds*Ip*Wp		
FpmaxASD =	1871	lbs	(0.7*Fpmax)	FpmaxASD =	2270 lbs
	(unit only)				(unit + upper rail)

Wind Loading - 2021 IBC/2022 CBC

Kz =	1.13		(For 60 ft roof height, Exposure C - Table 26.10-1 ACSE 7-16)
Kzt =	1.00		(Max. assumed topographic factor)
Kd =	0.85		(Directionality factor Table 26.6-1 ASCE 7-16)
Ke =	1.00		(Ground Elevation Factor Table 26.9-1 ASCE 7-16)
V =	110		(Wind velocity, mph for Occupancy Cat III-IV bldgs Exp. Cat C, Fig 26.5-1D - ASCE7-16)
$GCr_{(horiz)}$ =	1.9		(Refer Sect 29.4.1 ASCE 7-16)
$GCr_{(vert)}$ =	1.5		(Refer Sect 29.4.1 ASCE 7-16)
qz	29.8	psf	= $0.00256*Kz*Kzt*Kd*Ke*V^2$ (Eq. 26.10-1 ASCE 7-16)
$F_{h\ ASD\ trans}$ =	658	lbs	= 0.6*qz*GCr*Lunit*(Hunit+Hcurb) (Eq. 29.4-2)
$F_{h\ ASD\ long}$ =	459	lbs	= 0.6*qz*GCr*Wunit*(Hunit+Hcurb)
$F_{vert\ ASD}$ =	341	lbs	= 0.6*qz*GCr*Lunit*Wunit (Eq. 29.4-3)

Upper Curb Loading

Transverse:

$Compression_{SEISMIC}$ =	1891	lbs	=[FpmaxASD*Hcm+2*(1+0.14S_{DS})*Wtmax*wcurb]/wcurb
$Tension_{SEISMIC}$ =	1416	lbs	=[FpmaxASD*Hcm-2*(0.6-0.14S_{DS})*Wtmin*wcurb)]/wcurb
$Compression_{WIND}$ =	537	lbs	=[$F_{h\ ASD\ trans}$*Hcm+2*0.6*Wtmax*wcurb-$F_{vert\ ASD}$*wcurb/2]/wcurb
$Tension_{WIND}$ =	557	lbs	=[$F_{h\ ASD\ trans}$*Hcm-2*0.6*Wtmin*wcurb+$F_{vertASD}$*wcurb/2]/wcurb

---> Negative values indicate opposite load.

Longitudinal:

$Compression_{SEISMIC}$ =	1398	lbs	=[FpmaxASD*Hcm+2*(1+0.14*S_{DS})*Wtmax*Lcurb]/Lcurb
$Tension_{SEISMIC}$ =	924	lbs	=[FpmaxASD*Hcm-2*(0.6-0.14S_{DS})*Wtmin*Lcurb)]/Lcurb
$Compression_{WIND}$ =	259	lbs	=[$F_{h\ ASD\ long}$*Hcm+2*0.6*Wtmax*Lcurb-$F_{vertASD}$*Lcurb/2]/Lcurb
$Tension_{WIND}$ =	279	lbs	=[$F_{h\ ASD\ long}$*Hcm-2*0.6*Wtmin*Lcurb+$F_{vertASD}$*Lcurb/2]/Lcurb

---> Negative values indicate opposite load.

Governing Reactions:

Transverse:	$Comp_{MAX}$ =	1891	lbs	---> Along long edge of curb.
(on long edge)	$Tens_{MAX}$ =	1416	lbs	---> Along long edge of curb.
Longitudinal:	$Comp_{MAX}$ =	1398	lbs	---> Along short edge of curb.
(on short edge)	$Tens_{MAX}$ =	924	lbs	---> Along short edge of curb.

---> Negative values indicate opposite load.

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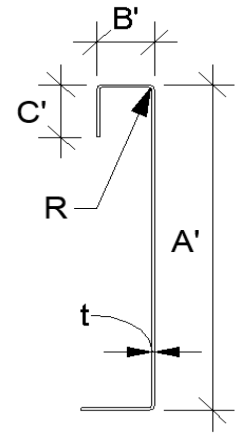
Curb Design

Fy =	50 ksi		Fu =	65 ksi	
E =	29500 ksi		t =	0.0713	14 Gauge

Calculate Section Properties of Curb

A'=	5.500	in	a =	5.144 in	= A'-(2r+t)
B'=	1.000	in	a'=	5.429 in	= A'-t
C'=	0.000	in (0 if no lips)	b =	0.822 in	= B'-[r+t/2+α(r+t/2)]
α =	0.000	(0 - no Lip; 1 w/ lip)	b'=	0.964 in	= B'-(t/2+αt/2)
R =	0.1069	(Inside bend radius)	c =	0.000 in	= α[C'-(r+t/2)]
t =	0.0713	in	c'=	0.000 in	= α(C'-t/2)
r'=	0.143	in = R+t/2	u =	0.224 in	= πr/2
x =	0.129	in (Distance between centroid and web centerline)			
Ix =	1.899	in^4	rx =	1.92 in	
Iy =	0.034	in^4	ry =	0.257 in	
A =	0.52	in^2	rmin =	0.257 in	



Axial Compression

Pa =	0.935 k	(Max Axial Comp)
Pn/Ωc =	7.976 k	
Fe =	35.72 ksi	
λc =	1.18	
Fn =	27.83 ksi	
Ly =	29.00 in	Lateral unbraced length
k_yL_y/r_y =	90	(assume k=0.8)

Ω_c = 1.80

$$\frac{P_n}{\Omega_c} = \frac{F_n A}{\Omega_c} \quad If\ \lambda_c \le 1.5;\ F_n = \left(0.658^{\lambda_c^2}\right) F_y \quad If\ \lambda_c > 1.5;\ F_n = \frac{0.877}{\lambda_c^2} F_y \quad \lambda_c = \sqrt{\frac{F_y}{F_e}} \quad F_e = \frac{\pi^2 E}{\left(kl/r\right)^2}$$

Compression Check = O.K.

Check Web Crippling

h =	5.5 in	-- Check limits:		C = 7.50
t =	0.0713 in	h/t =	77.14 ≤ 260	C_R = 0.08
N =	7.00	N/t =	98.18 ≤ 210	C_N = 0.12
Ω_w =	1.75	N/h =	1.273 ≤ 2.0	C_h = 0.048
P_n =	1.947 k	R/t =	1.50 ≤ 12.0	
P_n/Ω_w =	1.112 k			

(See table C3.4.1-2, fastened to support, two flange, end loading)

$$P_n = Ct^2F_y\sin(90)\left(1 - C_R\sqrt{\frac{R}{t}}\right)\left(1 + C_N\sqrt{\frac{N}{t}}\right)\left(1 - C_h\sqrt{\frac{h}{t}}\right)$$

Long side: Pu_{Trans} =	1.891 k	**web stiffener REQ'D**	# clips = 1
Short side: Pu_{Long} =	1.398 k	**web stiffener REQ'D**	# clips = 1

Check Web Stiffener 16Ga x 3/4in x 7in (C-channel)

$$P_n = 0.7\left(P_{wc} + A_eF_y\right) \ge P_{wc}$$

width of stiffener =	7.000 in	ts =	0.0566 16 Gauge	Pwc =	1.947 k
web of stiff. w =	6.717 in	Rs =	0.0849 in	Pn =	14.669 k
***Check w/ts ≤ 1.28√E/Fys		Ω_c =	1.70	Ae =	0.380 in^2
w/ts =	118.675				
1.28√(E/Fys) =	31.091	--> w/ts over limit	Use C3.7.2	Pn/Ω_c =	8.629 k **O.K.**

Corner Connections **1/4" ϕ SAE Grade 8 bolts w/ 1/4-20-UNC Threaded inserts**

Tcrnmax =	567 lbs	Max($F_{pmaxASD}$/4 -OR- $Fh_{ASDtrans}$/4 corner connections)
Vcrnmax =	946 lbs	Max(Tens/2 -OR- Comp/2 corner connections per side)

Bolt:	Tall =	2480 lbs	Vall =	1208 lbs
Threaded Insert:	Tall =	2860 lbs	Vall =	1096 lbs

# of Bolts required for Tension =	0.2
# of Bolts required for Shear =	0.9
# of Bolts Used =	2.0
Check Combined Stress in Bolts & Inserts:	0.546 **O.K.**

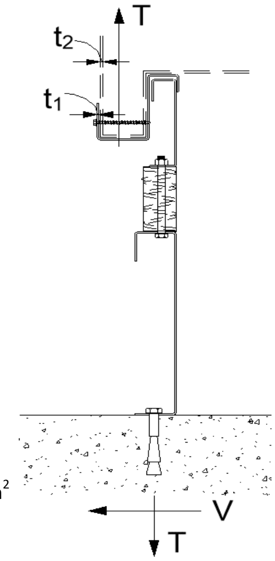
Check 1/8" welded connection <--- USE WELD Ω = 2.35

Assume L/t > 25: 25*t =	1.783 in
Lreq'd =	0.639 in

$$P_n/_\Omega = \frac{1}{\Omega}0.75tLF_u \ge V_{req} \qquad L_{req'd} = \frac{V_{req}\Omega}{0.75tF_u}$$

Connection Unit to Curb Clip — #10 SMS screw — Ω = 3.0

t1 =	0.0713 in (clip thickness)	t2/t1 = 1.0	Fu1 =	65 ksi
t2 =	0.0713 in (unit base rail thickness)		Fu2 =	65 ksi
d =	0.190 in (screw diameter)		dw =	0.375 in (nom. washer diameter)



For t2/t1 ≤ 1.0: Pns = 2266 #

Shear: $P_{ns} = 4.2F_{u2}\sqrt{t_2^3 d}$ — 2.27 k

Tension: $P_{ns} = 2.7t_1 dF_{u1}$ — 2.38 k

$P_{ns} = 2.7t_2 dF_{u2}$ — 2.38 k

For t2/t1 ≥ 2.5: Pns = 2377 #

$P_{ns} = 2.7t_1 dF_{u1}$ — 2.38 k

$P_{ns} = 2.7t_2 dF_{u2}$ — 2.38 k

Pns/Ω = 755 #
Pss/Ω = 540 # <- Controls
Pnot = 0.748 k (screw pull-out strength)
Pnov = 2.607 k (screw pull-over strength)
Pts/Ω = 249 # <- Controls
Pts/Ω = 820 # (full tensile screw capacity)

$P_{not} = 0.85t_c dF_{u2})$
$t_c = \min(t_1, t_2)$
$P_{nov} = 1.5t_1 d_w F_{u1}$

	Shear (k)	# clips	V_{clip} (k)	V_{allow}(lb)	# screws	spacing
Long side:	0.935	1	0.94	540 #	4	2.00 in
Short side:	0.935	1	0.94	540 #	4	2.00 in

clip width (in) = 7.00 — clip height = 2.5 in
min spacing = 0.57 in — edge distance = 0.5 in (min. 1.5d)

Check Block shear rupture: O.K. — thinnest part = 0.0713 — AISI BSR applies
Fy = 50 ksi — Ω = 2.22 bolt/screw connection
Agv = 0.463 in^2 — Anv = 0.416 in^2 — Ant = 0.082 in^2
Rn/Ω = 8.674 k — $R_n = 0.6F_y A_{gv} + F_u A_{nt} \leq 0.6F_u A_{nv} + F_u A_{nt}$

BSR O.K. (AISI Sect. E5.3)

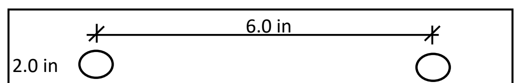
Curb Loads (copied from above)

Transverse: (on long edge)	$Comp_{MAX}$ =	2613	lbs
	$Tens_{MAX}$ =	2108	lbs
	$Shear_{MAX}$ =	2270	lbs
Longitudinal: (on short edge)	$Comp_{MAX}$ =	1881	lbs
	$Tens_{MAX}$ =	1375	lbs
	$Shear_{MAX}$ =	2270	lbs

Loads at each Isolator — Type: **CQA**

Transverse loading: (on long edge)	$Comp_{MAX}$ =	2613.2	lbs
	$Tens_{MAX}$ =	2107.6	lbs
# isolators: 1	$Shear_{MAX}$ =	567.4	lbs
Longitudinal loading: (on short edge)	$Comp_{MAX}$ =	1881.1	lbs
	$Tens_{MAX}$ =	1375.5	lbs
# isolators: 1	$Shear_{MAX}$ =	567.4	lbs

Max compression force on isolator: 2.613 k ≤ 3.176 k **O.K.**
Max uplift on isolator: 2.108 k ≤ 3.176 k **O.K.**
Max shear on isolator: 0.567 k ≤ 1.163 k **O.K.**



7.0 in

Forces on top bolt:
Tension = 2.108 k — d_b = 0.375 in
Shear = 0.567 k — upper rail, t = 0.0713 in

Shear on curb rail: $P_n = teF_u$ — Ω = 2.00 (Appendix A, Section E3.1 AISI)
Shear O.K. Pn/Ω = 4.635 k — e = 1.0 in

Net section rupture: $P_n = A_n F_t$ — Ω = 2.22 (Appendix A, Section E3.2 AISI)
Pn/Ω = 4.989 k — An = 0.116 in
N.S.R. O.K. — $F_t = (0.1 + 3d/s)F_u \leq F_u =$ 43.063 ksi

Bolt Bearing Strength: $P_n = Cm_f dtF_u$ — Ω = 2.50 (Section E3.3.1 AISI)
Pn/Ω = 2.086 k — d/t = 5.26
Bearing O.K. — C = 3.00 — mf = 1.00

Shear and tension in bolt: (Appendix A, Section E3.4 AISI)

Tension: $P_{nt} = A_b F_{nt}$ — Fnt = 40.5 ksi — A_b = 0.1104 in^2
Pnt/Ω = 1.988 k **No Good** — Ωt = 2.25 (Table E3.4-1, AISI)

Shear: $P_{nv} = A_b F_{nv}$ — Fnv = 24.0 ksi — Ωv = 2.40 (Table E3.4-1, AISI)
Pnv/Ω = 1.104 k **Bolt shear O.K.**

Combined Shear and tension in bolt:

$$F'_{nt} = 1.3F_{nt} - \frac{\Omega F_{nt}}{F_{nv}} f_v \leq F_{nt}$$

ft = 19.08 ksi — fv = 5.14 ksi — O.K.
F'nt = 31.84 ksi — Fnv/Ω = 10.00 ksi

$$P'_{nt} = A_b F'_{nt}$$

P'nt/Ω = 1.563 k **No Good - Use Welds**

Longitudinal weld loading: L = 1.5 — $P_n/\Omega = \frac{1}{\Omega}\left(1 - \frac{0.01L}{t_2}\right)Lt_2F_{u2} \geq V_{req}$ — Ω = 2.55
If L/t < 25: L/t = 21.04 — t = 0.0713 — Pn/Ω = 2.153 k

Transverse weld loading: t = 0.0713 — $P_n/\Omega = \frac{1}{\Omega}tLF_u \geq T_{req}$ — Ω = 2.35
L = 1 — Fu = 65 ksi — Pn/Ω = 1.972 k

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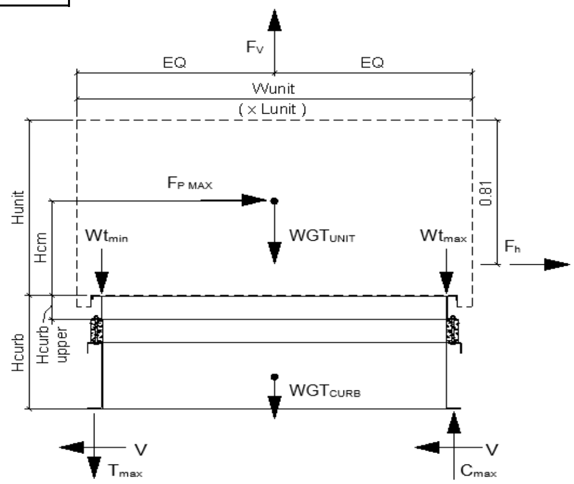
Client:	ProVent	PV2312		Base curb
Project:	CBISC-01	Iso Curb	CBISCLXS	
Unit:	ALL P***A CABINET			



Base Curb Information

Hbase curb = 25 in (Height of base curb)
Lcurb = 50.5 in (Length of base curb)
wcurb = 35 in (Width of base curb)
WGTbase = 128 lbs (Weight of base curb)
# Springs long side = 1 # Springs short side = 1

Unit Information

WGTunit = 521 lbs (Weight of Unit)
Wt'max = 184 lbs (Wtmax+1/4*WGTupper)
Wt'min = 138 lbs (Wtmin+1/4*WGTupper))
Hunit = 49 in (Height of unit above curb)
H'cm = 34.5 in (Hcm+10"(upper+spring))
Lunit = 51.25 in (Length of unit)
Wunit = 35.75 in (Width of unit)
$WGT_{unit+upper+base}$ = 760 lbs (Total weight)

Seismic Loading - 2021 IBC/2022CBC

Ss = 2.85 (Worst case for majority of California)
Fa = 1.20 (Default Site Class D - Table 11.4-1 ASCE 7-16)
Ip = 1.50 (Importance Factor Category III Building)
Sms = 3.420 (Fa*Ss) ap = 2.5
Sds = 2.280 (2/3*Sms) Rp = 2
Fpmax = 5.130 Wp (0.4*ap*Sds*Ip)*Wp*3/Rp <=1.6*Sds*Ip*Wp
FpmaxASD = 2270 lbs (0.7*Fpmax) FpmaxASD = 2729 lbs
(unit + upper rail) (unit + upper rail + base curb)

Wind Loading - 2021 IBC/2022 CBC

Kz = 1.13 (For 60 ft roof height, Exposure C - Table 26.10-1 ACSE 7-16)
Kzt = 1.00 (Max. assumed topographic factor)
Kd = 0.85 (Directionality factor Table 26.6-1 ASCE 7-16)
Ke = 1.00 (Ground Elevation Factor Table 26.9-1 ASCE 7-16)
V = 110 (Wind velocity, mph for Occupancy Cat III-IV bldgs Exp. Cat C, Fig 26.5-1D - ASCE7-16)
$GCr_{(horiz)}$ = 1.9 (Refer Sect 29.4.1 ASCE 7-16)
$GCr_{(vert)}$ = 1.5 (Refer Sect 29.4.1 ASCE 7-16)
qz 29.8 psf = 0.00256*Kz*Kzt*Kd*Ke*V^2 (Eq. 26.10-1 ASCE 7-16)
$F_{h\ ASD\ trans}$ = 1014 lbs = 0.6*qz*GCr*Lunit*(Hunit+Hbase curb+10") (Eq. 29.4-2)
$F_{h\ ASD\ long}$ = 707 lbs = 0.6*qz*GCr*Wunit*(Hunit+Hbase curb+10")
$F_{vert\ ASD}$ = 341 lbs = 0.6*qz*GCr*Lunit*Wunit (Eq. 29.4-3)

Base Curb Loading

Transverse:

$Compression_{SEISMIC}$ = 2723 lbs =[FpmaxASD*H'cm+2*(1+0.14S_{DS})*Wt'max*wcurb]/wcurb
$Tension_{SEISMIC}$ = 2159 lbs =[FpmaxASD*H'cm-2*(0.6-0.14S_{DS})*Wt'min*wcurb)]/wcurb
$Compression_{WIND}$ = 1050 lbs =[$F_{h\ ASD\ trans}$*H'cm+2*0.6*Wt'max*wcurb-$F_{vert\ ASD}$*wcurb/2]/wcurb
$Tension_{WIND}$ = 1004 lbs =[$F_{h\ ASD\ trans}$*H'cm-2*0.6*Wt'min*wcurb+$F_{vertASD}$*wcurb/2]/wcurb
---> Negative values indicate opposite load.

Longitudinal:

$Compression_{SEISMIC}$ = 2036 lbs =[FpmaxASD*H'cm+2*(1+0.14*S_{DS})*Wt'max*Lcurb]/Lcurb
$Tension_{SEISMIC}$ = 1473 lbs =[FpmaxASD*H'cm-2*(0.6-0.14S_{DS})*Wt'min*Lcurb)]/Lcurb
$Compression_{WIND}$ = 534 lbs =[$F_{h\ ASD\ long}$*H'cm+2*0.6*Wt'max*Lcurb-$F_{vertASD}$*Lcurb/2]/Lcurb
$Tension_{WIND}$ = 487 lbs =[$F_{h\ ASD\ long}$*H'cm-2*0.6*Wt'min*Lcurb+$F_{vertASD}$*Lcurb/2]/Lcurb
---> Negative values indicate opposite load.

Governing Reactions:

Transverse:	$Comp_{MAX}$ =	2723	lbs	---> Along long edge of curb.
(on long edge)	$Tens_{MAX}$ =	2159	lbs	---> Along long edge of curb.
Longitudinal:	$Comp_{MAX}$ =	2036	lbs	---> Along short edge of curb.
(on short edge)	$Tens_{MAX}$ =	1473	lbs	---> Along short edge of curb.

---> Negative values indicate opposite load.

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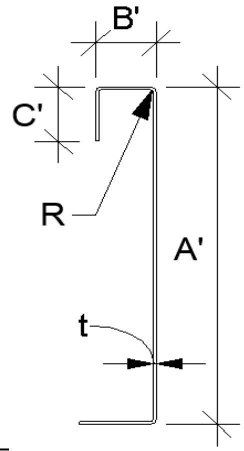
Curb Design

Fy = 50 ksi   Fu = 65 ksi
E = 29500 ksi   t = 0.0713 | 14 Gauge



Calculate Section Properties of Curb

A'=	25.000 in		a =	24.644 in	= A'-(2r+t)
B'=	1.750 in		a'=	24.929 in	= A'-t
C'=	0.000 in	(0 if no lips)	b =	1.572 in	= B'-[r+t/2+α(r+t/2)]
α =	0.000	(0 - no Lip; 1 w/ lip)	b'=	1.714 in	= B'-(t/2+αt/2)
R =	0.1069	(Inside bend radius)	c =	0.000 in	= α[C'-(r+t/2)]
t =	0.0713 in		c'=	0.000 in	= α(C'-t/2)
r'=	0.143 in	= R+t/2	u =	0.224 in	= πr/2
x =	0.104 in	(Distance between centroid and web centerline)			
Ix =	128.737 in		rx =	8.00 in	
Iy =	0.218 in		ry =	0.329 in	
A =	2.01 in^2		rmin =	0.329 in	

Axial Compression

Pu = 1.135 k   (Max Axial Comp) Ω_c = 1.80

Pn/Ωc = 18.917 k

Fe = 19.29 ksi

λc = 1.61

Fn = 16.91 ksi

Ly = 50.50 in   Lateral unbraced length

$k_y L_y / r_y$ = 123   (assume k=0.8)

$$\frac{P_n}{\Omega_c} = \frac{F_n A}{\Omega_c}$$

$$If\ \lambda_c \leq 1.5;\ F_n = \left(0.658^{\lambda_c^2}\right) F_y$$

$$If\ \lambda_c > 1.5;\ F_n = \frac{0.877}{\lambda_c^2} F_y$$

$$\lambda_c = \sqrt{\frac{F_y}{F_e}}$$

$$F_e = \frac{\pi^2 E}{\left(kl/r\right)^2}$$

Compression Check = O.K.

Check Web Crippling

h =	25 in	-- Check limits:		C = 4.00	(See table C3.4.1-2, fastened to support, one flange, end loading)
t =	0.0713 in	h/t =	350.63 ≤ 260	C_R = 0.14	
N =	7.00	N/t =	98.18 ≤ 210	C_N = 0.35	
Ω_w =	1.75	N/h =	0.28 ≤ 2.0	C_h = 0.02	
P_n =	2.105 k	R/t =	1.50 ≤ 9.0		
P_n/Ω_w =	1.203 k				

$$P_n = Ct^2 F_y \sin(90)\left(1 - C_R\sqrt{\frac{R}{t}}\right)\left(1 + C_N\sqrt{\frac{N}{t}}\right)\left(1 - C_h\sqrt{\frac{h}{t}}\right)$$

Long side: Pu_{Trans} = 2.723 k   **web stiffener REQ'D**   # clips = 1

Short side: Pu_{Long} = 2.036 k   **web stiffener REQ'D**   # clips = 1

*****h/t > 260; use web stiffeners**

Check Web Stiffener   16Ga x 1.5in x 7in (C-channel)

width of stiffener = 7.000 in   ts = 0.0566 | 16 Gauge

web of stiff. w = 6.717 in   Rs = 0.0849 in

***Check w/ts ≤ 1.28√E/Fys Ω_c = 1.70

w/ts = 118.675

1.28√(E/Fys) = 31.091   --> w/ts over limit   Use C3.7.2

$$P_n = 0.7\left(P_{wc} + A_e F_y\right) \geq P_{wc}$$

Pwc = 2.105 k   Ae = 0.380 in^2

Pn = 14.780 k

Pn/Ω_c = 8.694 k   **O.K.**

Corner Connections   **1/4" ϕ SAE Grade 8 bolts w/ 1/4-20-UNC Threaded inserts**

Tcrnmax = 682 lbs   Max($F_{pmaxASD}$/4 -OR- $Fh_{ASDtrans}$/4 corner connections)

Vcrnmax = 1361 lbs   Max(Tens/2 -OR- Comp/2 corner connections per side)

Bolt:	Tall =	2480 lbs	Vall =	1208 lbs
Threaded Insert:	Tall =	2860 lbs	Vall =	1096 lbs

# of Bolts required for Tension = 0.3

# of Bolts required for Shear = 1.2

# of Bolts Used = 3.0

Check Combined Stress in Bolts & Inserts: 0.506 **O.K.**

Check 1/8" welded connection   <--- USE WELD   Ω = 2.35

Assume L/t > 25: 25*t = 1.783 in

Lreq'd = 0.920 in

$$P_n/_{\Omega} = \frac{1}{\Omega} 0.75 t L F_u \geq V_{req}$$

$$L_{req'd} = \frac{V_{req}\Omega}{0.75 t F_u}$$

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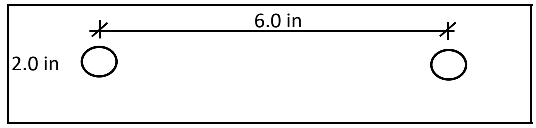
Curb Loads (copied from upper rail calcs)

Transverse: (on long edge)	$Comp_{MAX}$ =	2613	lbs
	$Tens_{MAX}$ =	2108	lbs
	$Shear_{MAX}$ =	2270	lbs
Longitudinal: (on short edge)	$Comp_{MAX}$ =	1881	lbs
	$Tens_{MAX}$ =	1375	lbs
	$Shear_{MAX}$ =	2270	lbs

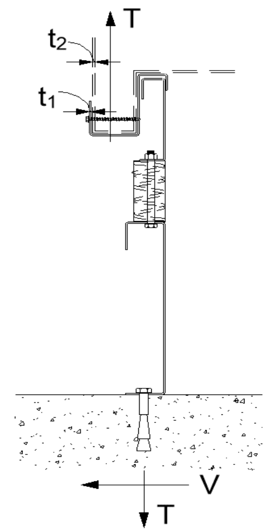
Loads at each Isolator Type: **CQA**

Transverse loading: (on long edge)	$Comp_{MAX}$ =	2613.2	lbs
	$Tens_{MAX}$ =	2107.6	lbs
# isolators: 1	$Shear_{MAX}$ =	567.4	lbs
Longitudinal loading: (on short edge)	$Comp_{MAX}$ =	1881.1	lbs
	$Tens_{MAX}$ =	1375.5	lbs
# isolators: 1	$Shear_{MAX}$ =	567.4	lbs

Max compression force on isolator: 2.613 k ≤ 3.176 k **O.K.**
Max uplift on isolator: 2.108 k ≤ 3.176 k **O.K.**
Max shear on isolator: 0.567 k ≤ 1.163 k **O.K.**



Forces on bottom bolts:
d_b = 0.5 in
base curb, t = 0.0713 in
Tension = 1.054 k / bolt
Shear = 0.284 k / bolt



Shear on base curb: $P_n = teF_u$ Ω = 2.00 (Appendix A, Section E3.1 AISI)
Pn/Ω = 4.635 k e = 1.0 in
Shear O.K.

Net section rupture: $P_n = A_nF_t$ Ω = 2.22 (Appendix A, Section E3.2 AISI)
Pn/Ω = 5.909 k An = 0.107 in
N.S.R. O.K. $F_t = (0.1 + 3d/s)F_u \le F_u =$ 55.250 ksi

Bolt Bearing Strength: $P_n = Cm_f dtF_u$ Ω = 2.50 (Section E3.3.1 AISI)
Pn/Ω = 2.781 k d/t = 7.01
Bearing O.K. C = 3.00 mf = 1.00

Shear and tension in bolt: (Appendix A, Section E3.4 AISI)
Tension $P_{nt} = A_bF_{nt}$ Fnt = 45.0 ksi A_b = 0.1963 in^2
Pnt/Ω = 3.927 k **Bolt tension O.K.** Ωt = 2.25
Shear $P_{nv} = A_bF_{nv}$ Fnv = 27.0 ksi Ωv = 2.40
Pnv/Ω = 2.209 k **Bolt shear O.K.** ***(Table E3.4-1, AISI)***

Combined Shear and tension in bolt:
$$F'_{nt} = 1.3F_{nt} - \frac{\Omega F_{nt}}{F_{nv}} f_v \le F_{nt}$$
ft = 10.73 ksi fv = 1.44 ksi O.K.
F'nt = 45.00 ksi Fnv/Ω = 11.25 ksi
$P'_{nt} = A_bF'_{nt}$ P'nt/Ω = 3.927 k **Combined Not Applicable -> F'nt = Fnt**

Connection of Curb to Supporting Structure

Roof Loading SEISMIC: $(0.6\text{-}0.14S_{DS})D + 0.7E$ WIND: 0.6D + W

Transverse: $Uplift_{MAX}$ = 4533 lbs $Shear_{MAX}$ = 1365 lbs

$Compression_{SEISMIC}$ =	5141 lbs	$=[FpmaxASD*(H'cm+Hbase\ curb)+(1+0.14S_{DS})*WGT_{unit+upper+base}*wcurb/2]/wcurb$
$Tension_{SEISMIC}$ =	4533 lbs	$=[FpmaxASD*(H'cm+Hbase\ curb)-(0.6-0.14S_{DS})*WGT_{unit+upper+base}*wcurb/2]/wcurb$
$Compression_{WIND}$ =	1781 lbs	$=[F_{h\ ASD\ trans}*(H'cm+Hbase\ curb)+0.6*WGT_{unit+upper+base}*wcurb/2-F_{vert\ ASD}*wcurb/2]/wcurb$
$Tension_{WIND}$ =	1666 lbs	$=[F_{h\ ASD\ trans}*(H'cm+Hbase\ curb)-0.6*WGT_{unit+upper+base}*wcurb/2+F_{vertASD}*wcurb/2]/wcurb$

Longitudinal: $Uplift_{MAX}$ = 3109 lbs $Shear_{MAX}$ = 1365 lbs

$Compression_{SEISMIC}$ =	3717 lbs	$=[FpmaxASD*(H'cm+Hbase\ curb)+(1+0.14S_{DS})*WGT_{unit+upper+base}*Lcurb/2]/Lcurb$
$Tension_{SEISMIC}$ =	3109 lbs	$=[FpmaxASD*(H'cm+Hbase\ curb)-(0.6-0.14S_{DS})*WGT_{unit+upper+base}*Lcurb/2]/Lcurb$
$Compression_{WIND}$ =	891 lbs	$=[F_{h\ ASD\ long}*(H'cm+Hbase\ curb)+0.6*WGT_{unit+upper+base}*Lcurb/2-F_{vert\ ASD}*Lcurb/2]/Lcurb$
$Tension_{WIND}$ =	776 lbs	$=[F_{h\ ASD\ long}*(H'cm+Hbase\ curb)-0.6*WGT_{unit+upper+base}*Lcurb/2+F_{vertASD}*Lcurb/2]/Lcurb$

Wood Attachment: **1/4"ϕ x 4.5" Simpson SDS screws w/ 2.75" threaded emb** (SGmin = 0.43)

Transverse:
$Tall_{metal}$ = 997 lbs $Vall_{metal}$ = 1097 lbs
$Tall_{wood}$ = 760 lbs $Vall_{wood}$ = 672 lbs

# of Screws Req'd for Uplift = 5.96 COMBINED LOADING: 0.902 O.K.
# of Screws Req'd for Shear = 2.03 Req'd Min Spacing = 6.07 in o.c.
Total # of screws required = 8

Use 8 - 1/4"ϕ x 4.5" Simpson SDS screws @ 6.1 in o.c. along long side of curb w/ 2.75" threaded embed

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Longitudinal:

# of Screws Req'd for Uplift =	4.09	COMBINED LOADING:	0.974 O.K.
# of Screws Req'd for Shear =	2.03	Screw Spacing =	6.75 in o.c.
Total # of screws required =	5		

Use 5 - 1/4"ϕ x 4.5" Simpson SDS screws @ 6.8 in o.c. along short side of curb w/ 2.75" threaded embed

Steel Deck Attachment: 1/2" ϕ A307 Bolts to steel angle below deck

	$T_{all,bolt}$ =	3927 lbs	$V_{all,bolt}$ =	2209 lbs
Transverse:	$T_{all,metal}$ =	2086 lbs	$V_{all,metal}$ =	2192 lbs
	# of Bolts Req'd for Uplift =	2.17	COMBINED LOADING:	0.849 O.K.
	# of Bolts Req'd for Shear =	0.62	Bolt Spacing =	19.25 in o.c.
	Total # of bolts required =	3		

Use 3 - 1/2" ϕ A307 Bolts to steel angle below deck @ 19.3 in o.c. along long side of curb

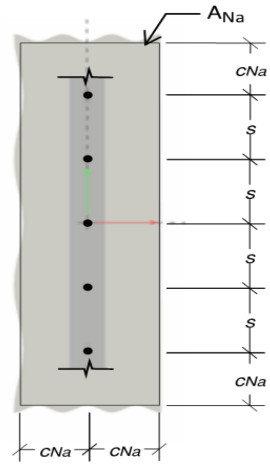
Longitudinal:

# of Bolts Req'd for Uplift =	1.49	COMBINED LOADING:	0.621 O.K.
# of Bolts Req'd for Shear =	0.62	Bolt Spacing =	23.00 in o.c.
Total # of bolts required =	2		

Use 2 - 1/2" ϕ A307 Bolts to steel angle below deck @ 23 in o.c. along short side of curb

For Concrete anchorage: SEISMIC $(0.6-0.14S_{DS})D + 0.7\Omega_o E$ Ωo = 2.5

Concrete Attachment: 0.625in ϕ HAS rods in Hilti HIT-HY 200 V3 epoxy w/ 4in embed



Epoxy: Hilti HIT-HY 200 V3 (ICC ESR 4868)

f'c = 4000 psi
h = 6 in (concrete thickness, t_min = h_ef + 2do) O.K.
h_ef = 4 in (effective embedment)
da = 0.625 in (anchor diameter) do = 0.75 in (hole diameter)
n = 5 (number of dummy anchors to check capacity with spacing effect)
s = 14 in (initial spacing estimate)
τk,cr / uncr = 1170 2220 psi (from ESR 4868, Table 14, Temp range B)
τk,cr / uncr = 1226 2327 psi If $f'_c > 2500$, multiply by $(f'_c/2500)^{0.1}$
c_{Na}= 9.0625 in (min. edge distance for full capacity); $c_{Na} = 10 d_a \sqrt{\frac{\tau_{uncr}}{1100}}$

Tension:
Bond strength
***Bond strength will govern over concrete breakout

$$N_{ag} = \frac{A_{Na}}{A_{Nao}} \varphi_{ec,Na} \varphi_{ed,Na} \varphi_{cp,Na} N_{ba}$$ (ACI318-14, 17.4.5.1b)

$\varphi_{ec,Na}\varphi_{ed,Na}\varphi_{cp,Na} = 1.0$

A_{Na}= 1343.52 in^2
A_{Nao}= 328.52 in^2
N_{ba} = 9535 lbs $N_{ba} = \lambda_a \tau_{cr} \pi d_a h_{ef} \alpha_{n,seismic}$ $\alpha_{n,seismic} = 0.99$
N_{ag} = 38995 lbs (group) $\lambda_a = 1.0$
$øN_{ag}$ = 19010 lbs (group) CONTROLS λ_a =1.0 for normal weight conc; 0.6 for lightw

Breakout strength

$$N_{cbg} = \frac{A_{Nc}}{A_{Nco}} \varphi_{ec,N} \varphi_{ed,N} \varphi_{cp,N} N_b \qquad N_b = \lambda_a k_c \sqrt{f'_c} h_{ef}^{1.5}$$

A_{Nc} = 816 in^2	N_b = 8601 lbs	$Ø_{conc}$ =	0.75
A_{Nco} = 144 in^2	kc = 17	$Ø_{bond}$ =	0.65
N_{cbg} = 48741 lbs (group)		$Ø_{seis}$ =	0.75
$øN_{cbg}$ = 27417 lbs (group)		$Ø_{steel}$ =	0.65

Shear:
Steel strength

Vsa,eq = 7865 (from ESR4868, Table 11) $\alpha_{v,seismic}$ = 0.6
øVsa,eq = 3067
$T_{all,LRFD}$ = 3802 lbs (anchor) $V_{all,LRFD}$ = 3067 lbs $\propto = (1+0.2SDS)D + 2.5E$ =1.421
$T_{all,ASD} = T_{all,LRFD}/\alpha$ = 2225 lbs $V_{all,ASD} = V_{all,LRFD}/\alpha$ = 1795 lbs D =0.758 E 0.242 $\propto$ =1.709

Transverse:	$Uplift_{MAX}$ = 5489 lbs	$Shear_{MAX}$ = 3411 lbs

$Compression_{SEISMIC}$ = 6078 lbs =[Ωo*FpmaxASD*(Hcm+Hcurb)+(1+0.14S_{DS})*$WGT_{unit+curb}$*wcurb/2]/wcurb
$Tension_{SEISMIC}$ = 5489 lbs =[Ωo*FpmaxASD*(Hcm+Hcurb)-(0.6-0.14S_{DS})*$WGT_{unit+curb}$*wcurb/2]/wcurb
$Shear_{SEISMIC}$ = 3411 lbs =Ωo*FpmaxASD/2

Min Bolts Req'd Uplift = 2.47 spacing = 19.25 in o.c. Tapplied = 1372.3 lbs
Min Bolts Req'd Shear = 2.00 spacing = 38.50 in o.c. Vapplied = 568.6 lbs

Try using	4	bolts
spaced at	12.83	in o.c.

COMBINED LOADING = $\frac{T_{applied}}{T_{allow,ASD}} + \frac{V_{apllied}}{V_{allow,ASD}} \leq 1.2$ = 0.93 O.K.

Use 4 - 0.625in ϕ HAS rods in Hilti HIT-HY 200 V3 epoxy @ 12.8 in o.c. max. along long side of curb w/ 4in embed

Longitudinal:	$Uplift_{MAX}$ = 3773 lbs	$Shear_{MAX}$ = 3411 lbs

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$Compression_{SEISMIC}$ =	4362	lbs	=[Ωo*FpmaxASD*(Hcm+Hcurb)+(1+0.14S_{DS})*$WGT_{unit+curb}$*Lcurb/2]/Lcurb
$Tension_{SEISMIC}$ =	3773	lbs	=[Ωo*FpmaxASD*(Hcm+Hcurb)-(0.6-0.14S_{DS})*$WGT_{unit+curb}$*Lcurb/2]/Lcurb
$Shear_{SEISMIC}$ =	3411	lbs	=Ωo*FpmaxASD/2

Min Bolts Req'd Uplift =	1.70	spacing =	11.50	in o.c.	Tapplied =	1886.4	lbs
Min Bolts Req'd Shear =	2.00	spacing =	23.00	in o.c.	Vapplied =	568.6	lbs

Try using 2 bolts
spaced at 23.00 in o.c.

COMBINED LOADING = $\frac{T_{applied}}{T_{allow,ASD}} + \frac{V_{apllied}}{V_{allow,ASD}} \leq 1.2$ = 1.16 O.K.

Use 2 - 0.625in ϕ HAS rods in Hilti HIT-HY 200 V3 epoxy @ 23 in o.c. max. along short side of curb w/ 4in embed

CURB DESIGN SUMMARY:	CBISC-01	CBISCLXS	**Unit:** ALL P***A CABINET
UPPER CURB RAIL THICKNESS:	0.0713 in	14 Gauge	
UNIT CLIP THICKNESS:	0.0713 in	14 Gauge	
# OF CLIPS (LONG SIDE) -	1 clips with 4 - #10 SMS screws each clip		
WEB STIFFENER:	16Ga x 3/4in x 7in (C-channel) stiffener at each clip		
# OF CLIPS (SHORT SIDE) -	1 clips with 4 - #10 SMS screws each clip		
WEB STIFFENER:	16Ga x 3/4in x 7in (C-channel) stiffener at each clip		
VIBRATION ISOLATOR TYPE:	CQA	Top stud diameter: 3/8	(1) - CQA Isolators long side
Anchor bolt diameter:	1/2	Anchor hole diamter: 9/16	(1) - CQA Isolators short side
BASE CURB THICKNESS:	0.0713 in	14 Gauge	***Must weld top of CQA***
WEB STIFFENER:	16Ga x 1.5in x 7in (C-channel) stiffener at each clip on base curb		
CORNER CONNECTION:	Use minimum 3 - 1/4" ϕ SAE Grade 8 bolts w/ 1/4-20-UNC Threaded inserts		

CURB ANCHORAGE	WOOD 1/4"ϕ x 4.5" Simpson SDS screws w/ 2.75" threaded embed (SGmin =	STEEL 1/2" ϕ A307 Bolts to steel angle below deck	CONCRETE 0.625in ϕ HAS rods in Hilti HIT-HY 200 V3 epoxy w/ 4in embed
LONG DIRECTION	8 @ 6.07 in o.c.	3 @ 19.25 in o.c.	4 @ 12.83 in o.c.
SHORT DIRECTION	5 @ 6.75 in o.c.	2 @ 23 in o.c.	2 @ 23 in o.c.