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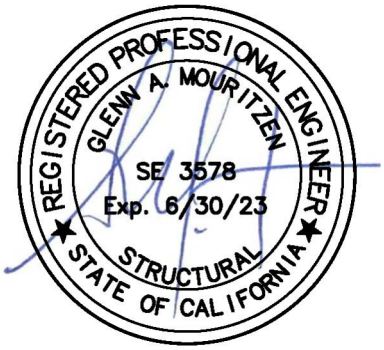
6593 Riverdale St.
San Diego, CA 92120
619-727-4800

Structural Calculations

for

CBKD-135 Series

KDKITSLU180



Prepared for:

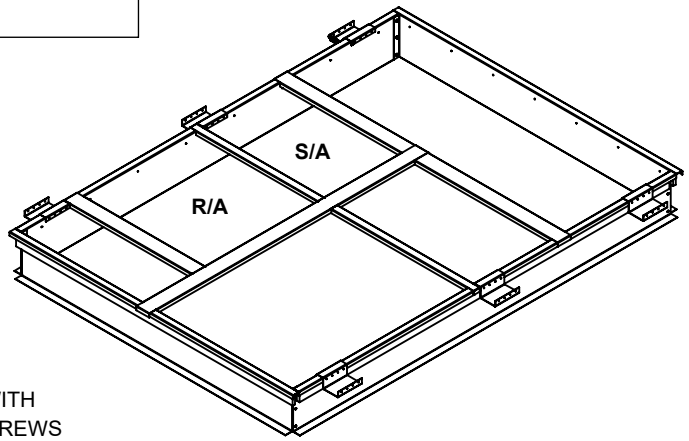
PROVENT / RRS

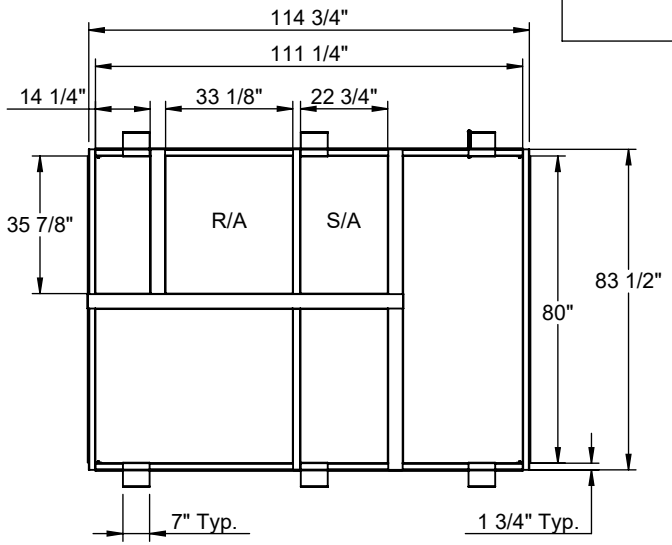
3847 Wabash Drive
Mira Loma, CA 91725

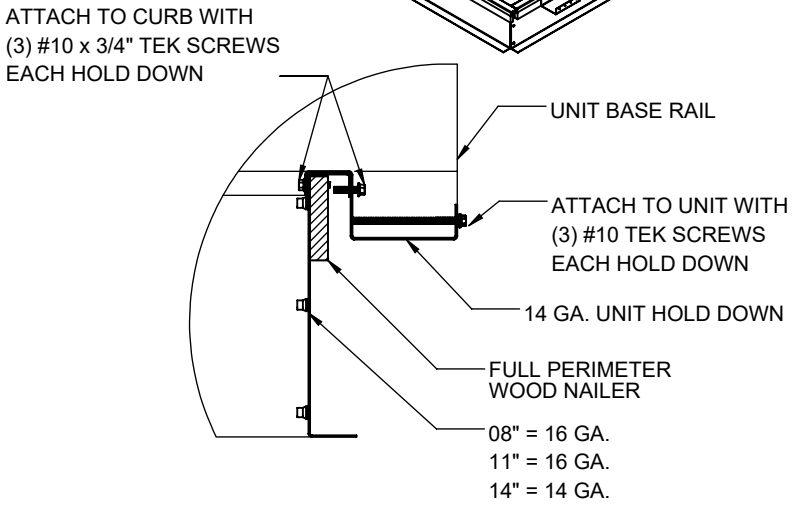
Date: October 1, 2021

Project Number: PV2101

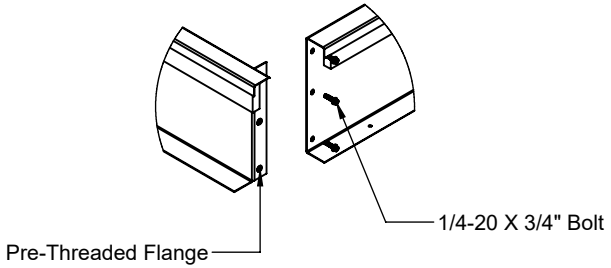
For wood, concrete and steel attachments see Roof Anchorage Detail, Form Nos. CB-60.	**STRUCTURALLY CALCULATED KNOCK-DOWN ROOF CURBS FOR YORK UNITS**	**PROVENT P/N**	**A**	**EST. WEIGHT**	**SEISMIC KIT P/N**
		CBKDSLU18008	8"	115 Lbs.	KDKITSLU180
		CBKDSLU18011	11"	129 Lbs.	
Will conform to seismic code requirements for knock-down or pre-assembled application. (Contact factory for assembled version.)	ZF 180	CBKDSLU18014	14"	144 Lbs	**Meets seismic requirements for the following codes:** CBC 2019 IBC 2018







HOLD DOWN DETAIL



CROSS SECTION DETAIL

REGISTERED PROFESSIONAL ENGINEER
GLENN A. MOURITZEN
SE 3578
Exp. 6/30/23
STRUCTURAL
STATE OF CALIFORNIA

ProVent

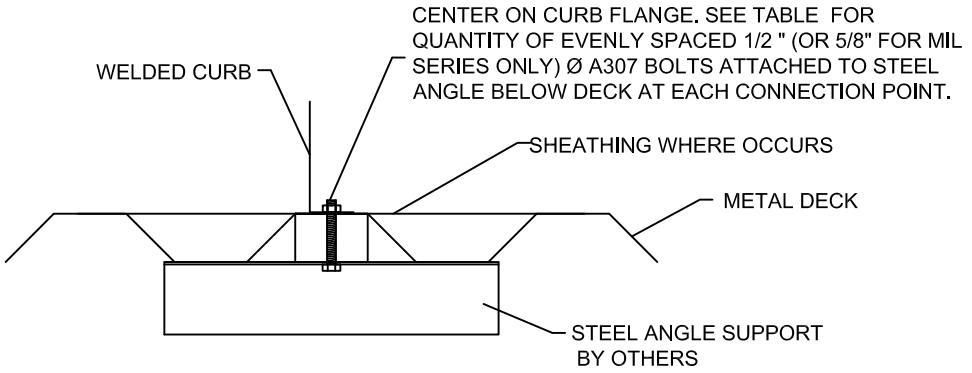
3847 WABASH DRIVE
MIRA LOMA, CA 91725

PHONE (951) 685-1101
FAX (619) 872-9799

SUBMITED TO:
COMPANY:
JOB NAME:
EQUIPMENT:
NOTES:

FORM NO:	**PART NUMBER:**	
CBKD-135	KDKITSLU180	
DATE:	**REV:**	**DRAWN BY:**
9/9/2021	**1**	ALL

STEEL ATTACHMENT



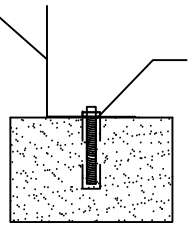
	NO. OF ANCHORAGE BOLTS REQUIRED	
CURB	LONG SIDE	SHORT SIDE
LXS	2 @ 34.5" O.C.	2 @ 19" O.C.
LXL	2 @ 34.5" O.C.	2 @ 29" O.C.
SUN3672	2 @ 60.5" O.C.	2 @ 39" O.C.
PRD3715	2 @ 68.88" O.C.	2 @ 39" O.C.
PRS	2 @ 58.88" O.C.	2 @ 28.69" O.C.
PRL	2 @ 72" O.C.	2 @ 41.5" O.C.
SLU180	3 @ 51.38" O.C.	2 @ 71.5" O.C.
SLM1830	3 @ 56.88" O.C	3 @ 35.75" O.C.

Meets seismic requirements for the following codes:
CBC 2019
IBC 2018

ROOF ANCHORAGE DETAIL	
CBKD Series	CBWC Series
LXS	LXS
LXL	LXL
SUN3672	SUN3672
PRD3715	PRD3715
PRS	PRS
PRL	PRL
SLU180	SLU180
SLM1830	SLM1830

ASSUMES:
CONC SLAB
f'c= 4000PSI MINIMUM
6" MIN THICKNESS
NORMAL WEIGHT CONCRETE
OR SAND LIGHT WEIGHT

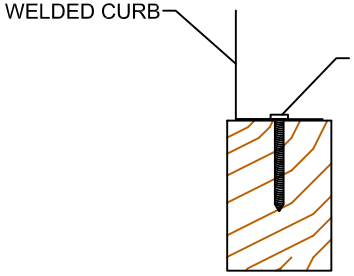
CONCRETE ATTACHMENT



	NO. OF ANCHORAGE BOLTS REQUIRED	
CURB	LONG SIDE	SHORT SIDE
LXS	4 @ 11.5" O.C.	3 @ 9.5" O.C.
LXL	4 @ 11.5" O.C.	3 @ 14.5" O.C.
SUN3672	4 @ 20.17" O.C.	3 @ 12.38" O.C.
PRD3715	9 @ 8.61" O.C.	7 @ 6.5" O.C.
PRS	5 @ 14.72" O.C.	4 @ 9.56" O.C.
PRL	6 @ 14.4" O.C.	5 @ 10.38" O.C.
SLU180	8 @ 14.68" O.C.	7 @ 11.92" O.C.
SLM1830	12 @ 10.34" O.C.	10 @ 7.94" O.C.

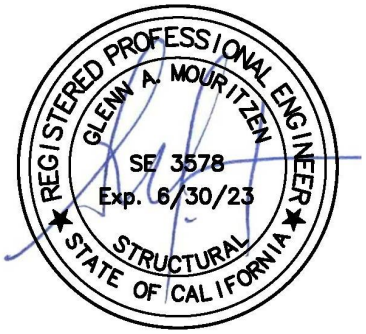
*** SIX INCHES FROM EACH CORNER EVENLY SPACED.**
**** CENTERED.**

WOOD ATTACHMENT



	NO. OF ANCHORAGE SCREWS REQUIRED	
CURB	LONG SIDE	SHORT SIDE
LXS	4 @ 12.83" O.C.	3 @ 11.5" O.C.
LXL	4 @ 12.83" O.C.	3 @ 16.5" O.C.
SUN3672	4 @ 21.5" O.C.	3 @ 14.38" O.C.
PRD3715	9 @ 9.11" O.C.	8 @ 6.14" O.C.
PRS	4 @ 20.96" O.C.	3 @ 16.34" O.C.
PRL	5 @ 19" O.C.	4 @ 15.17" O.C.
SLU180	9 @ 13.34" O.C.	7 @ 12.58" O.C.
SLM1830	13 @ 9.81" O.C.	12 @ 6.86" O.C.

FOUR INCHES FROM EACH CORNER EVENLY SPACED



ProVent
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SUBMITTED TO: _____
COMPANY: _____
JOB NAME: _____
EQUIPMENT: _____
NOTES: _____

FORM NO: CB-60

DATE:	REV:	DRAWN BY:
10/07/2021	7	FMM

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San Diego, CA 92120
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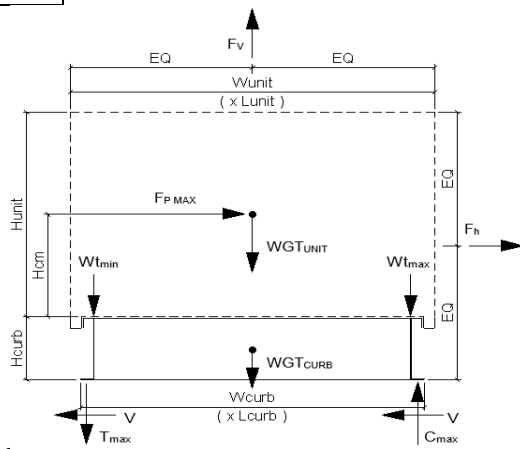
Client:	ProVent	PV2101	Previous: PV1806
Description:	CBKD-135	14"	KDKITSLU180
Unit:	ZF 180		

Curb Information

Hcurb =	14	in	(Height of curb)
Lcurb =	114.75	in	(Length of curb)
wcurb =	83.5	in	(Width of curb)
WGTcurb =	192	lbs	(Weight of curb)
# Clips long side =	3	# Clips short side =	0

Unit Information

WGTunit =	2057	lbs	(Weight of Unit)
Wtmax =	498	lbs	(Maximum corner weight)
Wtmin =	438	lbs	(Minimum corner weight)
Hunit =	48.625	in	(Height of unit above curb)
Hcm =	24.3125	in	(Height to center of mass)
Lunit =	125.25	in	(Length of unit)
Wunit =	92	in	(Width of unit)



Seismic Loading - 2018 IBC/2019 CBC

Ss =	2.85		(Worst case for majority of California)
Fa =	1.2		Default Site Class D
Ip =	1.50		(Importance Factor Category III Building)
Sms =	3.420		(Fa*Ss) ap = 2.5
Sds =	2.280		(2/3*Sms) Rp = 6
Fpmax =	1.710	Wp	(0.4*ap*Sds*Ip)*Wp*3/Rp <=1.6*Sds*Ip*Wp
FpmaxASD =	2462	lbs	(0.7*Fpmax) FpmaxASD = 2692 lbs
	(unit only)		(unit and curb)

Wind Loading - 2018 IBC/2019 CBC

*** Exposure Category C ***

Kz =	1.13		(For 60 ft roof height, Exposure C - Table 26.10-1 ACSE 7-16)
Kzt =	1.0		(No topographic effects assumed for rooftop mounted units)
Kd =	0.85		(Directionality factor Table 26.6-1 ASCE 7-16)
V =	115		(Max wind velocity, mph for Cat III & IV bldgs Exp. Cat C)
$GCr_{(horiz)}$ =	1.9		(Refer Sect 29.4.1 ASCE 7-16)
$GCr_{(vert)}$ =	1.5		(Refer Sect 29.4.1 ASCE 7-16)
qz	32.5	psf	$= 0.00256*Kz*Kzt*Kd*V^2$ (Eq. 26.10-1 ASCE 7-16)
$F_{h\ ASD\ trans}$ =	2019	lbs	= 0.6*qz*GCr*Lunit*(Hunit+Hcurb) (Eq. 29.4-2)
$F_{h\ ASD\ long}$ =	1483	lbs	= 0.6*qz*GCr*Wunit*(Hunit+Hcurb)
$F_{vert\ ASD}$ =	2342	lbs	= 0.6*qz*GCr*Lunit*Wunit (Eq. 29.4-3)

Curb Loading

Transverse:

$Compression_{SEISMIC}$ =	2031	lbs	$=[FpmaxASD*Hcm+2*(1+0.14S_{DS})*Wtmax*wcurb]/wcurb$
$Tension_{SEISMIC}$ =	471	lbs	$=[FpmaxASD*Hcm-2*(0.6-0.14S_{DS})*Wtmin*wcurb]/wcurb$
$Compression_{WIND}$ =	15	lbs	$=[F_{h\ ASD\ trans}*Hcm+2*0.6*Wtmax*wcurb-F_{vert\ ASD}*wcurb/2]/wcurb$
$Tension_{WIND}$ =	1233	lbs	$=[F_{h\ ASD\ trans}*Hcm-2*0.6*Wtmin*wcurb+F_{vertASD}*wcurb/2]/wcurb$

---> Negative values indicate opposite load.

Longitudinal:

$Compression_{SEISMIC}$ =	1836	lbs	$=[FpmaxASD*Hcm+2*(1+0.14*S_{DS})*Wtmax*Lcurb]/Lcurb$
$Tension_{SEISMIC}$ =	276	lbs	$=[FpmaxASD*Hcm-2*(0.6-0.14S_{DS})*Wtmin*Lcurb]/Lcurb$
$Compression_{WIND}$ =	-259	lbs	$=[F_{h\ ASD\ long}*Hcm+2*0.6*Wtmax*Lcurb-F_{vertASD}*Lcurb/2]/Lcurb$
$Tension_{WIND}$ =	960	lbs	$=[F_{h\ ASD\ long}*Hcm-2*0.6*Wtmin*Lcurb+F_{vertASD}*Lcurb/2]/Lcurb$

---> Negative values indicate opposite load.

Governing Reactions:

Transverse:	$Comp_{MAX}$ =	2031	lbs	---> Along long edge of curb.
(on long edge)	$Tens_{MAX}$ =	1233	lbs	---> Along long edge of curb.
Longitudinal:	$Comp_{MAX}$ =	1836	lbs	---> Along short edge of curb.
(on short edge)	$Tens_{MAX}$ =	960	lbs	---> Along short edge of curb.

---> Negative values indicate opposite load.

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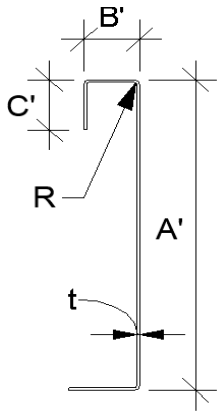
Curb Design



Fy =	50 ksi	Fu =	65 ksi		
E =	29500 ksi	t =	0.0713	14 Gauge	

Calculate Section Properties of Curb

A' =	14.000 in	a =	13.644 in	= A'-(2r+t)	
B' =	1.750 in	a' =	13.929 in	= A'-t	
C' =	0.000 in (0 if no lips)	b =	1.572 in	= B'-[r+t/2+α(r+t/2)]	
α =	0.000 (0 - no Lip; 1 w/ lip)	b' =	1.714 in	= B'-(t/2+αt/2)	
R =	0.1069 (Inside bend radius)	c =	0.000 in	= α[C'-(r+t/2)]	
t =	0.0713 in	c' =	0.000 in	= α(C'-t/2)	
r' =	0.143 in = R+t/2	u =	0.224 in	= πr/2	
x =	0.171 in (Distance between centroid and web centerline)				
Ix =	27.499 in^4	rx =	4.73 in		
Iy =	0.204 in^4	ry =	0.407 in		
A =	1.23 in^2	rmin =	0.407 in		

Axial Compression

Pu =	1.231 k	(Max Axial Comp)	Ωc = 1.80
Pn/Ωc =	17.057 k		
Fe =	30.16 ksi	$\frac{P_n}{\Omega_c} = \frac{F_n A}{\Omega_c}$	If $\lambda_c \le 1.5$; $F_n = \left(0.658^{\lambda_c{}^2}\right) F_y$
λc =	1.29		If $\lambda_c > 1.5$; $F_n = \frac{0.877}{\lambda_c{}^2} F_y$
Fn =	24.98 ksi	$\lambda_c = \sqrt{\frac{F_y}{F_e}}$	$F_e = \frac{\pi^2 E}{\left(kl/r\right)^2}$
Ly =	50 in	Lateral unbraced length	
$k_y L_y/r_y$ =	98	(assume k=0.8)	

Compression Check = **O.K.**

Check Web Crippling

h =	14 in	-- Check limits:		C = 4.00	(See table C3.4.1-2, fastened to support, one flange, end loading)
t =	0.0713 in	h/t =	196.35 ≤ 200	C_R = 0.14	
N =	7.00	N/t =	98.18 ≤ 210	C_N = 0.35	
Ω_w =	1.75	N/h =	0.5 ≤ 2.0	C_h = 0.02	
P_n =	2.422 k	R/t =	1.50 ≤ 9.0		
P_n/Ω_w =	1.384 k				
Long side: Pu_{Trans} =	0.677 k	**O.K.**	# clips = 3		
Short side: Pu_{Long} =	0.918 k	**O.K.**	# clips = 2		

$$P_n = Ct^2 F_y \sin(90)\left(1 - C_R\sqrt{\frac{R}{t}}\right)\left(1 + C_N\sqrt{\frac{N}{t}}\right)\left(1 - C_h\sqrt{\frac{h}{t}}\right)$$

Check Web Stiffener

16Ga x 3/4" x 7" (C-channel)

width of stiffener =	7.000 in	ts =	0.0566	16 Gauge
web of stiff. w =	6.717 in	Rs =	0.0849 in	
***Check w/ts ≤ 1.28√E/Fys		Ωc =	1.70	
w/ts =	118.675			
1.28√(E/Fys) =	31.091	--> w/ts over limit	Use C3.7.2	

$$P_n = 0.7\left(P_{wc} + A_e F_y\right) \ge P_{wc}$$

Pwc =	2.422 k	Ae =	0.380 in^2
Pn =	15.002 k	Pn/Ω =	8.825 k

Not Req'd

Corner Connections

1/4" φ SAE Grade 8 bolts w/ 1/4-20-UNC Threaded inserts

Tcrnmax =	673 lbs	Max($F_{pmaxASD}$/4 -OR- $Fh_{ASDtrans}$/4 corner connections)			
Vcrnmax =	1015 lbs	Max(Tens/2 -OR- Comp/2 corner connections per side)			
Bolt:	Tall =	2480 lbs	Vall =	1208 lbs	
Threaded Insert:	Tall =	2860 lbs	Vall =	1536 lbs	

# of Bolts required for Tension =	0.3
# of Bolts required for Shear =	0.8
# of Bolts Used =	2.0
Check Combined Stress in Bolts & Inserts:	0.556 **O.K.**

Check 1/8" welded connection

<--- USE WELD Ω = 2.35

Assume L/t > 25: 25*t =	1.783 in	$P_n/\Omega = \frac{1}{\Omega} 0.75 t L F_u \ge V_{req}$	$L_{req'd} = \frac{V_{req}\Omega}{0.75 t F_u}$
Lreq'd =	0.687 in		

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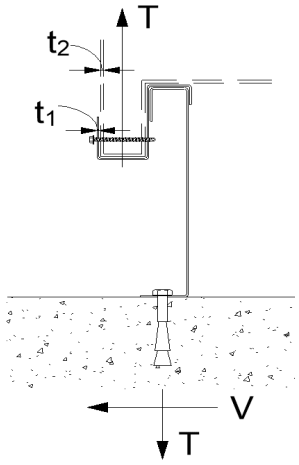
Connection Unit to Curb Clip | #10 SMS screw | Ω = 3.0

t1 = 0.0713 in | Fu1 = 65 ksi

t2 = 0.1017 in (unit base rail thickness) | Fu2 = 65 ksi

d = 0.190 in (screw diameter) | dw = 0.375 in (nom. washer diameter)

t2/t1 = 1.4



For t2/t1 ≤ 1.0: | Pns = 2377 # | For t2/t1 ≥ 2.5: | Pns = 2377 #

Shear: $P_{ns} = 4.2F_{u2}\sqrt{t_2^3 d}$ 3.86 k

$P_{ns} = 2.7t_1 dF_{u1}$ 2.38 k | $P_{ns} = 2.7t_1 dF_{u1}$ 2.38 k

$P_{ns} = 2.7t_2 dF_{u2}$ 3.39 k | $P_{ns} = 2.7t_2 dF_{u2}$ 3.39 k

Pns/Ω = 792 #

Pss/Ω = 540 # <- Controls

Tension: Pnot = 1.068 k (screw pull-out strength)

$P_{not} = 0.85t_c dF_{u2})$

$t_c = \min(t_1, t_2)$

Pnov = 2.607 k (screw pull-over strength)

$P_{nov} = 1.5t_1 d_w F_{u1}$

Pts/Ω = 356 # <- Controls

Pts/Ω = 820 # (full tensile screw capacity)

	Shear (k)	# clips	V_{clip} (k)	V_{allow}(lb)	# screws	spacing
Long side:	2.462	3	0.82	540 #	2	6.00 in
Short side:	2.462	2	1.23	540 #	3	3.00 in

clip width (in) = 7.00 | clip height = 1.4 in

min spacing = 0.57 in | edge distance = 0.5 in (min. 1.5d)

Check Block shear rupture: O.K. | thinnest part = 0.0713 AISI BSR applies

Fy = 50 ksi | Ω = 2.22 bolt/screw connection

Agv = 0.463 in² | Anv = 0.430 in² | Ant = 0.042 in²

Rn/Ω = 7.500 k

$R_n = 0.6F_y A_{gv} + F_u A_{nt} \leq 0.6F_u A_{nv} + F_u A_{nt}$

BSR O.K. (AISI Sect. E5.3)

Connection of Curb to Supporting Structure

Roof Loading SEISMIC: (0.6-0.14S_{DS})D + 0.7E WIND: 0.6D + W

Transverse:	$Uplift_{MAX}$ = 2087 lbs	$Shear_{MAX}$ = 1346 lbs	
$Compression_{SEISMIC}$ =	2719 lbs	=[FpmaxASD*(Hcm+Hcurb)+(1+0.14S_{DS})*$WGT_{unit+curb}$*wcurb/2]/wcurb	
$Tension_{SEISMIC}$ =	2087 lbs	=[FpmaxASD*(Hcm+Hcurb)-(0.6-0.14S_{DS})*$WGT_{unit+curb}$*wcurb/2]/wcurb	
$Compression_{WIND}$ =	430 lbs	=[$F_{h\ ASD\ trans}$*(Hcm+Hcurb)+0.6*$WGT_{unit+curb}$*wcurb/2-$F_{vert\ ASD}$*wcurb/2]/wcurb	
$Tension_{WIND}$ =	1423 lbs	=[$F_{h\ ASD\ trans}$*(Hcm+Hcurb)-0.6*$WGT_{unit+curb}$*wcurb/2+$F_{vertASD}$*wcurb/2]/wcurb	
Longitudinal:	$Uplift_{MAX}$ = 1751 lbs	$Shear_{MAX}$ = 1346 lbs	
$Compression_{SEISMIC}$ =	2382 lbs	=[FpmaxASD*(Hcm+Hcurb)+(1+0.14S_{DS})*$WGT_{unit+curb}$*Lcurb/2]/Lcurb	
$Tension_{SEISMIC}$ =	1751 lbs	=[FpmaxASD*(Hcm+Hcurb)-(0.6-0.14S_{DS})*$WGT_{unit+curb}$*Lcurb/2]/Lcurb	
$Compression_{WIND}$ =	-1 lbs	=[$F_{h\ ASD\ long}$*(Hcm+Hcurb)+0.6*$WGT_{unit+curb}$*Lcurb/2-$F_{vert\ ASD}$*Lcurb/2]/Lcurb	
$Tension_{WIND}$ =	991 lbs	=[$F_{h\ ASD\ long}$*(Hcm+Hcurb)-0.6*$WGT_{unit+curb}$*Lcurb/2+$F_{vertASD}$*Lcurb/2]/Lcurb	

Wood Attachment: **1/4"φ x 3.5" Simpson SDS screws w/ 2.25" threaded emb** (SGmin = 0.43)

$Tall_{metal}$ = 997 lbs | $Vall_{metal}$ = 1097 lbs

Transverse: $Tall_{wood}$ = 616 lbs | $Vall_{wood}$ = 672 lbs

# of Screws Req'd for Uplift = 3.39 | COMBINED LOADING: 0.770 O.K.

# of Screws Req'd for Shear = 2.00 | Screw Spacing = 17.8 in o.c.

Total # of screws Required = 7

1/4"φ x 3.5" Simpson SDS screws @ 17.8 in o.c. along long side of curb w/ 2.25" threaded embed

Longitudinal:

# of Screws Req'd for Uplift = 2.8 | COMBINED LOADING: 0.692 O.K.

# of Screws Req'd for Shear = 2.0 | Screw Spacing = 12.6 in o.c.

Total # of screws Required = 7

1/4"φ x 3.5" Simpson SDS screws @ 12.6 in o.c. along short side of curb w/ 2.25" threaded embed

Steel Deck Attachment: **1/2" φ A307 Bolts to steel angle below deck**

$Tall_{bolt}$ = 3927 lbs | $Vall_{bolt}$ = 2209 lbs

Transverse: $Tall_{metal}$ = 2086 lbs | $Vall_{metal}$ = 2192 lbs

# of Bolts Req'd for Uplift = 1.00 | COMBINED LOADING: 0.231 O.K.

# of Bolts Req'd for Shear = 0.61 | Bolt Spacing = 51.4 in o.c.

Total # of Bolts Required = 3

1/2" φ A307 Bolts to steel angle below deck @ 51.4 in o.c. along long side of curb

Longitudinal:

# of Bolts Req'd for Uplift = 0.84 | COMBINED LOADING: 0.375 O.K.

# of Bolts Req'd for Shear = 0.61 | Req'd Min Spacing = 71.5 in o.c.

Total # of Bolts Required = 2

1/2" φ A307 Bolts to steel angle below deck @ 71.5 in o.c. along short side of curb

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For Concrete anchorage: SEISMIC $(0.6-0.14S_{DS})D + 0.7\Omega_o E$ $\Omega o = 2.0$

Concrete Attachment: 3/4" φ thrd'd rods in Hilti Hit-HY 200 epoxy w/ 4" embed

$Tall_{LRFD}$ = 1722 lbs $Vall_{LRFD}$ = 2032 lbs $\propto = (1 + 0.2SDS)D + 2.5E = 1.708$

$Tall_{ASD} = Tall_{LRFD}/\alpha$ = 920.9 lbs $Vall_{ASD} = Vall_{LRFD}/\alpha$ = 1086.6 lbs $(D = 0.758, E = 0.242)$

Transverse: $Uplift_{MAX}$ = 2155 lbs $Shear_{MAX}$ = 2692 lbs

$Compression_{SEISMIC}$ = 3954 lbs =[Ωo*FpmaxASD*(Hcm+Hcurb)+(1+0.14S_{DS})*$WGT_{unit+curb}$*wcurb/2]/wcurb

$Tension_{SEISMIC}$ = 2155 lbs =[Ωo*FpmaxASD*(Hcm+Hcurb)-(0.6-0.14S_{DS})*$WGT_{unit+curb}$*wcurb/2]/wcurb

$Shear_{SEISMIC}$ = 2692 lbs =Ωo*FpmaxASD/2

Min Bolts Req'd Uplift = 2.34 spacing = 51.38 in o.c. Tapplied = 307.8 lbs

Min Bolts Req'd Shear = 2.48 spacing = 51.38 in o.c. Vapplied = 207.1 lbs

Try using 7 bolts
spaced at 17.13 in o.c.

COMBINED LOADING = $\frac{T_{applied}}{T_{allow,ASD}} + \frac{V_{apllied}}{V_{allow,ASD}} \leq 1.2$ = 0.52

Use 7 - 3/4" ϕ thrd'd rods in Hilti Hit-HY 200 epoxy @ 17.1 in o.c. max. along long side of curb w/ 4" embed

Longitudinal: $Uplift_{MAX}$ = 1482 lbs $Shear_{MAX}$ = 2692 lbs

$Compression_{SEISMIC}$ = 3281 lbs =[Ωo*FpmaxASD*(Hcm+Hcurb)+(1+0.14S_{DS})*$WGT_{unit+curb}$*Lcurb/2]/Lcurb

$Tension_{SEISMIC}$ = 1482 lbs =[Ωo*FpmaxASD*(Hcm+Hcurb)-(0.6-0.14S_{DS})*$WGT_{unit+curb}$*Lcurb/2]/Lcurb

$Shear_{SEISMIC}$ = 2692 lbs =Ωo*FpmaxASD/2

Min Bolts Req'd Uplift = 1.61 spacing = 35.75 in o.c. Tapplied = 247.0 lbs

Min Bolts Req'd Shear = 2.48 spacing = 35.75 in o.c. Vapplied = 207.1 lbs

Try using 6 bolts
spaced at 14.30 in o.c.

COMBINED LOADING = $\frac{T_{applied}}{T_{allow,ASD}} + \frac{V_{apllied}}{V_{allow,ASD}} \leq 1.2$ = 0.46

Use 6 - 3/4" ϕ thrd'd rods in Hilti Hit-HY 200 epoxy @ 14.3 in o.c. max. along short side of curb w/ 4" embed

CURB DESIGN SUMMARY:	CBKD-135	KDKITSLU180	Unit:	ZF 180
CURB RAIL THICKNESS:	0.0713 in	14 Gauge		
UNIT CLIP THICKNESS:	0.0713 in	14 Gauge		
# OF CLIPS (LONG SIDE) -	3 clips with 2 - #10 SMS screws each clip			
WEB STIFFENER:	NOT REQUIRED			
# OF CLIPS (SHORT SIDE) -	2 clips with 3 - #10 SMS screws each clip			
WEB STIFFENER:	NOT REQUIRED			
CORNER CONNECTION:	Use 2 - 1/4" ϕ SAE Grade 8 bolts w/ 1/4-20-UNC Threaded inserts			

CURB ANCHORAGE	WOOD	STEEL	CONCRETE
	1/4"φ x 3.5" Simpson SDS screws w/ 2.25" threaded embed	1/2" φ A307 Bolts to steel angle below deck	3/4" φ thrd'd rod in Hilti HIT-HY 200 epoxy, min. 4" embed
LONG DIRECTION	7 @ 17.79 in o.c.	3 @ 51.38 in o.c.	7 @ 17.13 in o.c.
SHORT DIRECTION	7 @ 12.58 in o.c.	2 @ 71.5 in o.c.	6 @ 14.3 in o.c.