MOUR GROUP
ENGINEERING + DESIGN

6593 Riverdale St.
San Diego, CA 92120

619-727-4800

Structural Calculations

for

CBKD Series Roof Curbs

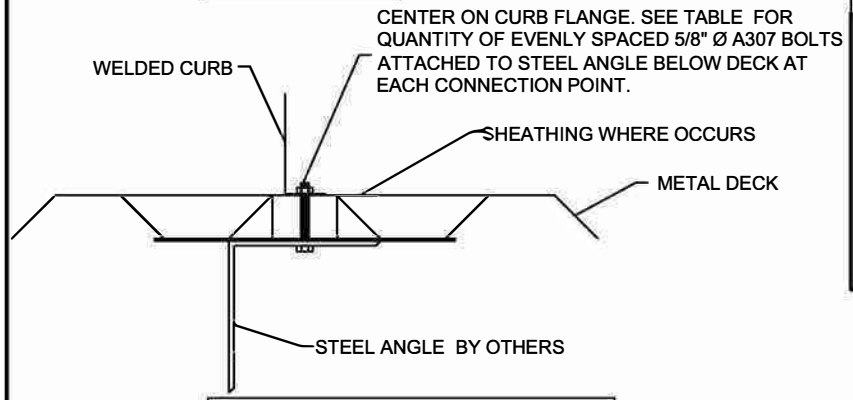
CBKD-92A (80-265-19)
2020 Florida Building Code requirements

Prepared for:

PROVENT
3847 Wabash Drive
Mira Loma, CA 91725

Date: May 19, 2021
Project Number: PV2101

STEEL ATTACHMENT



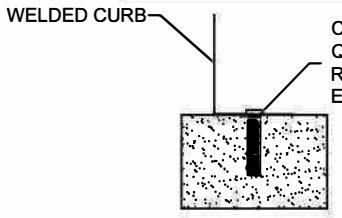
	NO. OF ANCHORAGE BOLTS REQUIRED		
CURB KIT	LONG SIDE *	SHORT SIDE *	UNIT
80-265-49	2 @ 34.5" o.c.	2 @ 19" o.c.	LXS
80-265-50	2 @ 34.5" o.c.	2 @ 29" o.c.	LXL
80-265-13	2 @ 61" o.c.	2 @ 25.25" o.c.	SUNLINE 3-6 TON
80-265-45	2 @ 58.38" o.c.	2 @ 28.19" o.c.	PRESTIGE SMALL
80-265-46	2 @ 72" o.c.	2 @ 41" o.c.	PRESTIGE LARGE
80-265-29	3 @ 34.69" o.c.	2 @ 39.5" o.c.	PREDATOR
80-265-19	3 @ 51.63" o.c.	2 @ 72" o.c.	SUNLINE ULTRA
80-265-18	3 @ 57.13" o.c.	2 @ 72" o.c.	SUNLINE MAGNA

WIND LOAD ROOF ANCHORAGE DETAIL

Meets wind requirements for the following codes:
FBC 2020
based on ASCE 7-16.

Wind:
190 mph exposure D category III or IV
building, max BLDG height: 60 ft
Kzt=1.00 max

CONCRETE ATTACHMENT

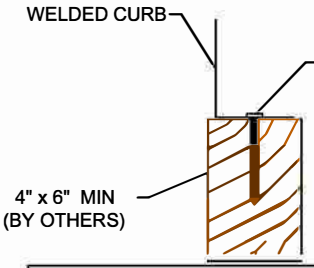


- NORMAL WEIGHT CONC SLAB
- f'c=4000 PSI MIN
- 6" MIN THICK CONC.
- SPECIAL INSPECTION REQUIRED (ESR-3187)

	NO. OF ANCHORAGE BOLTS REQUIRED		
CURB KIT	LONG SIDE *	SHORT SIDE **	UNIT
80-265-49	5 @ 8.63" o.c.	3 @ 9.5" o.c.	LXS
80-265-50	5 @ 8.63" o.c.	4 @ 9.67" o.c.	LXL
80-265-13	5 @ 15.25" o.c.	3 @ 12.63" o.c.	SUNLINE 3-6 TON
80-265-45	5 @ 14.59" o.c.	3 @ 14.09" o.c.	PRESTIGE SMALL
80-265-46	5 @ 18" o.c.	4 @ 13.67" o.c.	PRESTIGE LARGE
80-265-29	9 @ 8.67" o.c.	5 @ 9.88" o.c.	PREDATOR
80-265-19	8 @ 14.75" o.c.	6 @ 14.4" o.c.	SUNLINE ULTRA
80-265-18	12 @ 10.39" o.c.	8 @ 10.29" o.c.	SUNLINE MAGNA

STEEL AND CONCRETE ANCHORS ARE 6" FROM EACH CORNER EVENLY SPACED

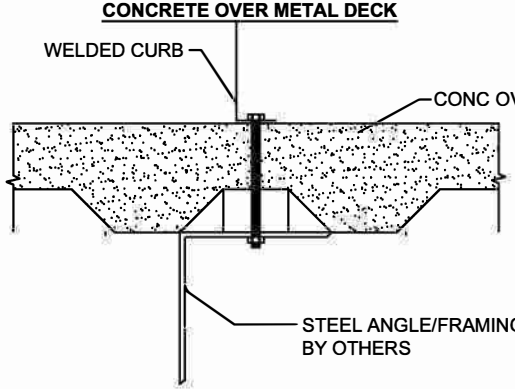
WOOD ATTACHMENT



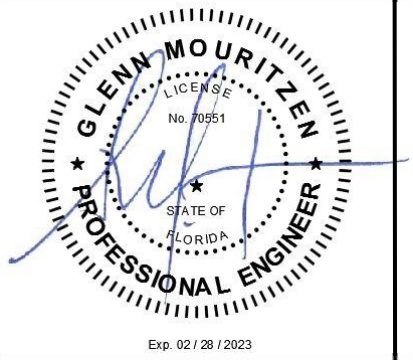
(SPECIFIC GRAVITY OF WOOD= 0.43 MIN)

	NO. OF ANCHORAGE SCREWS REQUIRED		
CURB KIT	LONG SIDE	SHORT SIDE	UNIT
80-265-49	6 @ 7.7" o.c.	3 @ 11.5" o.c.	LXS
80-265-50	5 @ 9.63" o.c.	5 @ 8.25" o.c.	LXL
80-265-13	6 @ 13" o.c.	3 @ 14.63" o.c.	SUNLINE 3-6 TON
80-265-45	6 @ 12.48" o.c.	3 @ 16.09" o.c.	PRESTIGE SMALL
80-265-46	6 @ 15.2" o.c.	4 @ 15" o.c.	PRESTIGE LARGE
80-265-29	10 @ 8.15" o.c.	6 @ 8.7" o.c.	PREDATOR
80-265-19	8 @ 15.32" o.c.	6 @ 15.2" o.c.	SUNLINE ULTRA
80-265-18	14 @ 9.1" o.c.	8 @ 10.86" o.c.	SUNLINE MAGNA
	FOUR INCHES FROM EACH CORNER EVENLY SPACED.		

CONCRETE OVER METAL DECK



NOTE: FOR CONC OVER METAL DECK OVER STEEL FRAMING USE STEEL ATTACHMENT



Exp. 02 / 28 / 2023

RRS ROOFTOP SYSTEMS

1625 DIPLOMAT DRIVE
CARROLTON, TX 75006

PHONE (972) 247-7447
FAX (972) 243-0940

SUBMITTED TO: _____
COMPANY: _____
JOB NAME: _____
EQUIPMENT: _____
NOTES: _____

FORM NO:		
CB-25A		
DATE:	REV:	DRAWN BY:
3/29/2021	2	ALL

For wood, concrete and steel attachments see Roof Anchorage Detail, Form No. CB-25A.

Will conform to wind load code requirements for knock-down or pre-assembled application. (Contact factory for assembled version.)

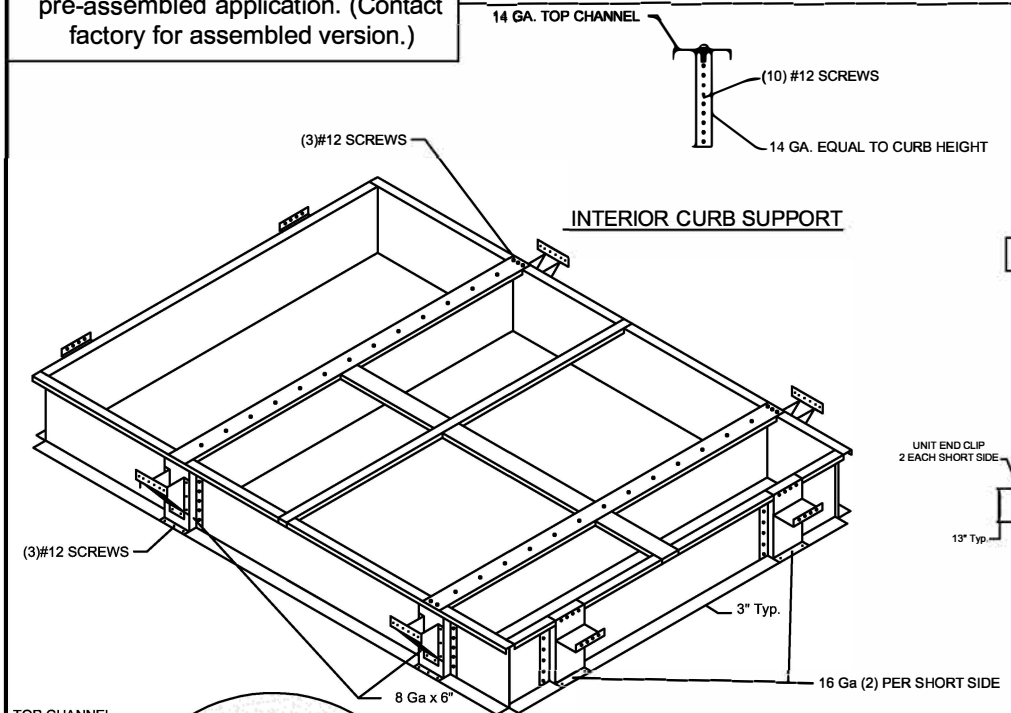
CALCULATED WIND ROOF CURBS FOR YORK UNITS
SERIES 20/ LARGE SUNLINE- SHORT RAIL

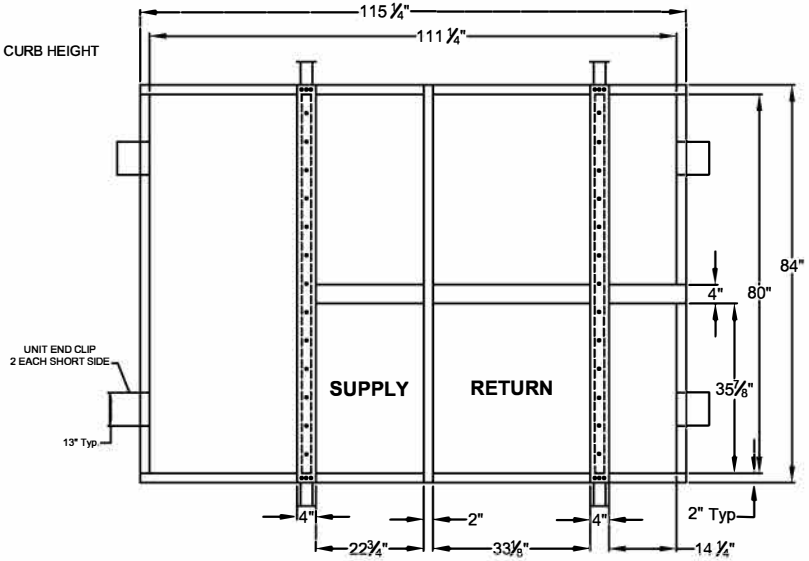
ZF 180

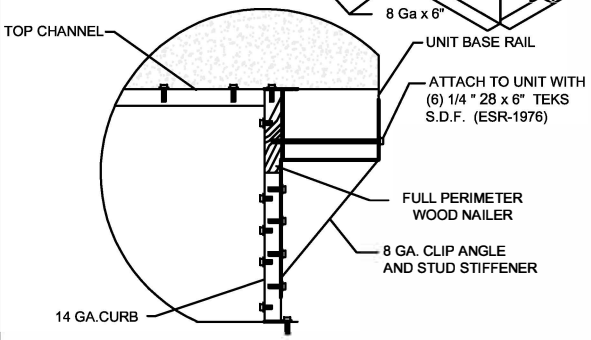
ProVent P/N	A	WEIGHT
80-265-1914	14"	328 Lbs
80-265-1918	18"	426 Lbs

Meets wind requirements for the following codes:
FBC 2020
based on ASCE 7-16.

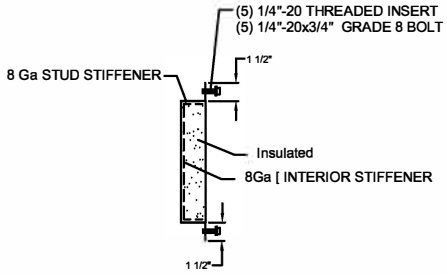
Wind:
190 mph exposure D category III or IV building, max BLDG height: 60 ft
Kzt=1.00 max



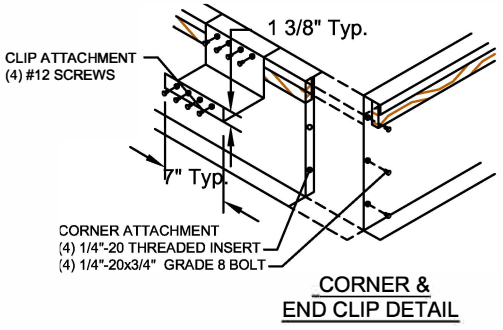




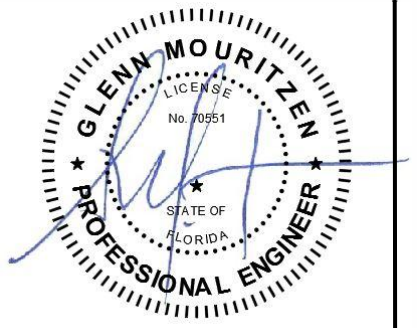
HOLD DOWN DETAIL



STUD STIFFENER



CORNER &
END CLIP DETAIL



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PHONE (972) 247-7447
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SUBMITTED TO: _____
COMPANY: _____
JOB NAME: _____
EQUIPMENT: _____
NOTES: _____

FORM NO:		PART NUMBER:
CBKD-92A		80-265-19
DATE: 3/29/2021	**REV:** 1	**DRAWN BY:** ALL

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San Diego, CA 92120
(619)727-4800

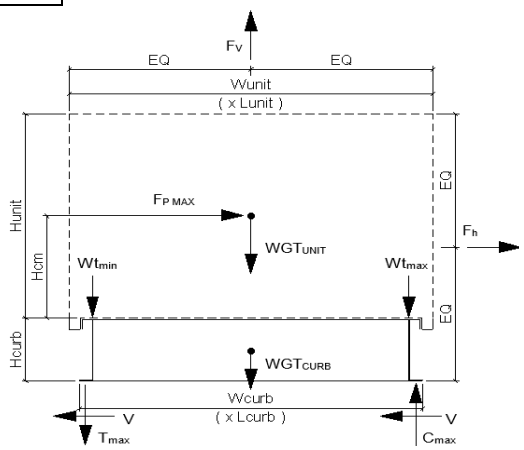
Client:	ProVent	PV2101	Previous: PV1807
Description:	CBKD-92	80-265-19**	
Unit:	Large Sunline: ZF 180		

Curb Information

Hcurb =	18	in	(Height of curb)
Lcurb =	115.25	in	(Length of curb)
wcurb =	84	in	(Width of curb)
WGTcurb =	426	lbs	(Weight of curb)
# Clips long side =	2	# Clips short side =	2

Unit Information

WGTunit =	1870	lbs	(Weight of Unit)
Wtmax =	498	lbs	(Maximum corner weight)
Wtmin =	438	lbs	(Minimum corner weight)
Hunit =	48.625	in	(Height of unit above curb)
Hcm =	24.3125	in	(Height to center of mass)
Lunit =	125.25	in	(Length of unit)
Wunit =	92	in	(Width of unit)



Seismic Loading - 2020 FBC/2018 IBC

Ss =	0.15		(Worst Case for state of Florida)
Fa =	2.4		(Worst case Site class E from Table 11.4-1 ASCE 7-16)
Sms =	0.360		(Fa*Ss)
Sds =	0.240		(2/3*Sms)
Ip =	1.5		(Importance Factor Category III or IV Building)
Fpmax =	0.576	WGTunit	(1.6*Sds*Ip)*WGTunit (Eq 13.3-2 ASCE 7-16)
FpmaxASD =	754	lbs	(0.7*Fpmax) FpmaxASD = 926 lbs
	(unit only)		(unit and curb)

Wind Loading - 2020 FBC/2018 IBC *** Exposure Category D ***

Kz =	1.31		(For 60 ft roof height, Exposure D - Table 26.10-1 ACSE 7-16)
Kzt =	1.00		(Max. assumed topographic factor)
Kd =	0.85		(Directionality factor Table 26.6-1 ASCE 7-16)
Ke =	1.00		(Ground Elevation Factor Table 26.9-1 ASCE 7-16)
V =	190		(Wind velocity, mph for Occupancy Cat III-IV bldgs Exp. Cat C, Fig 26.5-1D - ASCE7-16)
$GCr_{(horiz)}$ =	1.9		(Refer Sect 29.4.1 ASCE 7-16)
$GCr_{(vert)}$ =	1.5		(Refer Sect 29.4.1 ASCE 7-16)
qz	102.9	psf	$= 0.00256*Kz*Kzt*Kd*Ke*V^2$ (Eq. 26.10-1 ASCE 7-10)
$F_{h\ ASD\ trans}$ =	6798	lbs	= 0.6*qz*GCr*Lunit*(Hunit+Hcurb) (Eq. 29.4-2)
$F_{h\ ASD\ long}$ =	4993	lbs	= 0.6*qz*GCr*Wunit*(Hunit+Hcurb)
$F_{vert\ ASD}$ =	7411	lbs	= 0.6*qz*GCr*Lunit*Wunit (Eq. 29.4-3)

Curb Loading

Transverse:

$Compression_{SEISMIC}$ =	1248	lbs	$=[FpmaxASD*Hcm+2*(1+0.14S_{DS})*Wtmax*wcurb]/wcurb$
$Tension_{SEISMIC}$ =	189	lbs	$= Comp_{SEISMIC}-(0.6-0.14S_{DS})*WGTunit$
$Compression_{WIND}$ =	-1140	lbs	$=[F_{h\ transASD}*Hcm+2*0.6*Wtmax*wcurb-F_{vertASD}*wcurb/2]/wcurb$
$Tension_{WIND}$ =	5149	lbs	$= Comp_{WIND}+Fvert-0.6*WGTunit$

---> Negative values indicate opposite load.

Longitudinal:

$Compression_{SEISMIC}$ =	1189	lbs	$=[FpmaxASD*Hcm+2*(1+0.14*S_{DS})*Wtmax*Lcurb]/Lcurb$
$Tension_{SEISMIC}$ =	129	lbs	$= Comp_{SEISMIC}-(0.6-0.14S_{DS})*WGTunit$
$Compression_{WIND}$ =	-2055	lbs	$=[F_{h\ transASD}*Hcm+2*0.6*Wtmax*Lcurb-F_{vertASD}*Lcurb/2]/Lcurb$
$Tension_{WIND}$ =	4235	lbs	$= Comp_{WIND}+Fvert-0.6*WGTunit$

---> Negative values indicate opposite load.

Governing Reactions:

Transverse:	$Comp_{MAX}$ =	1248	lbs	---> Along long edge of curb.
(on long edge)	$Tens_{MAX}$ =	5149	lbs	---> Along long edge of curb.
Longitudinal:	$Comp_{MAX}$ =	1189	lbs	---> Along short edge of curb.
(on short edge)	$Tens_{MAX}$ =	4235	lbs	---> Along short edge of curb.

---> Negative values indicate opposite load.

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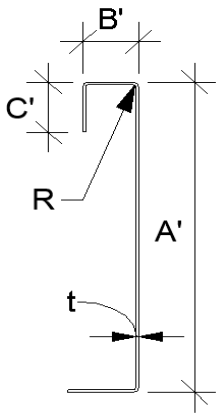
Curb Design

Fy = 50 ksi Fu = 65 ksi t = 0.0713 [14 Gauge]

E = 29500 ksi



Calculate Section Properties of Curb

A' =	[18.000] in		a =	17.644 in	= A'-(2r+t)
B' =	[2.000] in		a' =	17.929 in	= A'-t
C' =	[0.000] in	(0 if no lips)	b =	1.822 in	= B'-[r+t/2+α(r+t/2)]
α =	[0.000]	(0 - no Lip; 1 w/ lip)	b' =	1.964 in	= B'-(t/2+αt/2)
R =	0.1069	(Inside bend radius)	c =	0.000 in	= α[C'-(r+t/2)]
t =	0.0713 in		c' =	0.000 in	= α(C'-t/2)
r' =	0.143 in	= R+t/2	u =	0.224 in	= πr/2
x =	0.178 in	(Distance between centroid and web centerline)			
Ix =	56.073 in	(Moment of Inertia about X-Axis)			
Iy =	0.311 in	(Moment of Inertia about Y-Axis)			
A =	1.55 in^2				
rx =	6.02 in				
ry =	0.448 in				
rmin =	0.448 in				

Axial Compression

Pu = 3.399 k (Max Axial Comp) Ωc = 1.80

Pn/Ωc = 9.782 k

Fe = 12.95 ksi

$$\frac{P_n}{\Omega_c} = \frac{F_n A}{\Omega_c} \qquad If\ \lambda_c \le 1.5;\ F_n = \left(0.658^{\lambda_c{}^2}\right) F_y \qquad If\ \lambda_c > 1.5;\ F_n = \frac{0.877}{\lambda_c{}^2} F_y$$

$$\lambda_c = \sqrt{\frac{F_y}{F_e}} \qquad F_e = \frac{\pi^2 E}{\left(kl/r\right)^2}$$

λc = 1.96

Fn = 11.36 ksi

Ly = 84 in Lateral unbraced length

$k_y L_y/r_y$ = 150 (assume k=0.8)

Compression Check = O.K.

Check Web Crippling

h = 18 in -- Check limits: C = 4.00

t = 0.0713 in h/t = 252.45 ≤ 200 C_R = 0.14

N = 7.00 N/t = 98.18 ≤ 210 C_N = 0.35

Ω_w = 1.75 N/h = 0.388889 ≤ 2.0 C_h = 0.02

(See table C3.4.1-2, fastened to support, one flange, end loading)

P_n = 2.296 k R/t = 1.50 ≤ 9.0

P_n/Ω_w = 1.312 k

$$P_n = Ct^2 F_y \sin(90)\left(1 - C_R\sqrt{\frac{R}{t}}\right)\left(1 + C_N\sqrt{\frac{N}{t}}\right)\left(1 - C_h\sqrt{\frac{h}{t}}\right)$$

Long side: Pu_{Trans} = 0.624 k O.K. # clips = 2

Short side: Pu_{Long} = 0.594 k O.K. # clips = 2

*****h/t > 200; use web stiffeners**

Check Web Stiffener

16Ga x 3/4" x 7" (C-channel)

width of stiffener = 7.000 in ts = 0.0566 [16 Gauge]

web of stiff. w = 6.717 in Rs = 0.0849 in

***Check w/ts ≤ 1.28√E/Fys Ωc = 1.70

w/ts = 118.675

1.28√(E/Fys) = 31.091 --> w/ts over limit Use C3.7.2

$$P_n = 0.7\left(P_{wc} + A_e F_y\right) \ge P_{wc}$$

Pwc = 2.296 k Ae = 0.380 in^2

Pn = 14.913 k Pn/Ω = 8.773 k

O.K.

Corner Connections

1/4" φ SAE Grade 8 bolts w/ 1/4-20-UNC Threaded inserts

Tcrnmax = 1700 lbs Max($F_{pmaxASD}$/4 -OR- $Fh_{ASDtrans}$/4 corner connections)

Vcrnmax = 2574 lbs (Max Ten/2 corner connections per side)

Bolt:	Tall =	[2480] lbs	Vall =	[1096] lbs
Threaded Insert:	Tall =	[2860] lbs	Vall =	[1714] lbs

# of Bolts required for Tension = 0.7

# of Bolts required for Shear = 2.3 ***If combined fails:

# of Bolts Used = [3.0] USE --> 4.0

Check Combined Stress in Bolts & Inserts: 1.011 N.G. StressComb = 0.759 O.K.

Check 1/8" welded connection

<--- USE WELD Ω = 2.35

Assume L/t > 25: 25*t = 1.783 in $P_n/\Omega = \frac{1}{\Omega} 0.75 t L F_u \ge V_{req}$ $L_{req'd} = \frac{V_{req}\Omega}{0.75 t F_u}$

Lreq'd = 1.741 in

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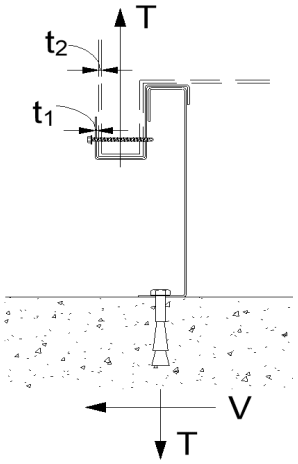
Connection Unit to Curb Clip | #12 SMS screw | Ω = 3.0

t1 = 0.0713 in | Fu1 = 65 ksi

t2 = 0.1017 in (unit base rail thickness) | Fu2 = 65 ksi

d = 0.216 in (screw diameter) | dw = 0.375 in (nom. washer diameter)

t2/t1 = 1.4



For t2/t1 ≤ 1.0: | Pns = 2703 #

Shear: $P_{ns} = 4.2F_{u2}\sqrt{t_2^3 d}$ = 4.12 k

$P_{ns} = 2.7t_1 dF_{u1}$ = 2.70 k

$P_{ns} = 2.7t_2 dF_{u2}$ = 3.86 k

Pns/Ω = 901 #

Pss/Ω = 840 # <- Controls

For t2/t1 ≥ 2.5: | Pns = 2703 #

$P_{ns} = 2.7t_1 dF_{u1}$ = 2.70 k

$P_{ns} = 2.7t_2 dF_{u2}$ = 3.86 k

Tension: Pnot = 1.214 k (screw pull-out strength)

Pnov = 2.607 k (screw pull-over strength)

$P_{not} = 0.85t_c dF_{u2})$

$t_c = \min(t_1, t_2)$

$P_{nov} = 1.5t_1 d_w F_{u1}$

Pts/Ω = 405 # <- Controls

Pts/Ω = 845 # (full tensile screw capacity)

	Shear (k)	# clips	V_{clip} (k)	V_{allow}(lb)	# screws	spacing
Long side:	2.497	2	1.25	840 #	2	6.00 in
Short side:	3.399	2	1.70	840 #	3	3.00 in

clip width (in) = 7.00 | clip height = 2.5 in

min spacing = 0.65 in | edge distance = 0.5 in (min. 1.5d)

Check Block shear rupture: O.K. | thinnest part = 0.0713 AISI BSR applies

Fy = 50 ksi | Ω = 2.22 bolt/screw connection

Agv = 0.463 in^2 | Anv = 0.425 in^2 | Ant = 0.081 in^2

Rn/Ω = 8.647 k | $R_n = 0.6F_y A_{gv} + F_u A_{nt} \leq 0.6F_u A_{nv} + F_u A_{nt}$ (AISI Sect. E5.3)

BSR O.K.

Connection of Curb to Supporting Structure

Roof Loading | SEISMIC: (0.6-0.14SDS)D + 0.7E | WIND: 0.6D + W

Transverse:	$Uplift_{MAX}$ = 6441 lbs	$Shear_{MAX}$ = 3399 lbs	
$Compression_{SEISMIC}$ =	1653 lbs	=[FpmaxASD*(Hcm+Hcurb)+(1+0.14S_{DS})*($WGT_{unit+curb}$/2)*wcurb]/wcurb	
$Tension_{SEISMIC}$ =	352 lbs	=$Comp_{SEISMIC}$-(0.6-0.14S_{DS})*(WGTunit+curb)	
$Compression_{WIND}$ =	408 lbs	=[$F_{h\ transASD}$*(Hcm+Hcurb)+0.6*($WGT_{unit+curb}$/2)*wcurb-$F_{vertASD}$*wcurb/2]/wcurb	
$Tension_{WIND}$ =	6441 lbs	=[$F_{h\ transASD}$*(Hcm+Hcurb)-0.6*($WGT_{unit+curb}$/2)*wcurb+$F_{vertASD}$*wcurb/2]/wcurb	
Longitudinal:	$Uplift_{MAX}$ = 4850 lbs	$Shear_{MAX}$ = 2497 lbs	
$Compression_{SEISMIC}$ =	1526 lbs	=[FpmaxASD*(Hcm+Hcurb)+(1+0.14S_{DS})*($WGT_{unit+curb}$/2)*Lcurb]/Lcurb	
$Tension_{SEISMIC}$ =	226 lbs	=$Comp_{SEISMIC}$-(0.6-0.14S_{DS})*(WGTunit+curb)	
$Compression_{WIND}$ =	-1183 lbs	=[$F_{h\ transASD}$*(Hcm+Hcurb)+0.6*($WGT_{unit+curb}$/2)*Lcurb-$F_{vertASD}$*Lcurb/2]/Lcurb	
$Tension_{WIND}$ =	4850 lbs	=[$F_{h\ transASD}$*(Hcm+Hcurb)-0.6*($WGT_{unit+curb}$/2)*Lcurb+$F_{vertASD}$*Lcurb/2]/Lcurb	

Wood Attachment: **Use 5/8" φ wood lag screws** **w/ 3.5" Min. Embed**

$Tall_{screw}$ = 6133 lbs | $Vall_{metal}$ = 2744 lbs

Transverse: $Tall_{wood}$ = 1196 lbs | $Vall_{wood}$ = 1024 lbs

# of Screws Req'd for Uplift = 5.39 | COMBINED LOADING: 0.910 O.K.

# of Screws Req'd for Shear = 3.32 | Screw Spacing = 15.3 in o.c.

Total # of screws Required = 8

Use 5/8" φ wood lag screws @ 15.3 in o.c. along long side of curb w/ 3.5" Min. Embed

Longitudinal:

# of Screws Req'd for Uplift = 4.1 | COMBINED LOADING: 0.850 O.K.

# of Screws Req'd for Shear = 2.4 | Screw Spacing = 15.2 in o.c.

Total # of screws Required = 6

Use 5/8" φ wood lag screws @ 15.2 in o.c. along short side of curb w/ 3.5" Min. Embed

Steel Deck Attachment: **Use 5/8" φ A307 Bolts attached to steel angle below deck**

$Tall_{bolt}$ = 6903 lbs | $Vall_{bolt}$ = 3682 lbs

Transverse: 6903 lbs | 3682 lbs

# of Bolts Req'd for Uplift = 0.93 | COMBINED LOADING: 0.283 O.K.

# of Bolts Req'd for Shear = 0.92 | Bolt Spacing = 51.6 in o.c.

Total # of Bolts Required = 3

Use 5/8" φ A307 Bolts attached to steel angle below deck @ 51.6 in o.c. along long side of curb

Longitudinal:

# of Bolts Req'd for Uplift = 0.70 | COMBINED LOADING: 0.340 O.K.

# of Bolts Req'd for Shear = 0.68 | Req'd Min Spacing = 72.0 in o.c.

Total # of Bolts Required = 2

Use 5/8" φ A307 Bolts attached to steel angle below deck @ 72 in o.c. along short side of curb

MOUR GROUP
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For Concrete anchorage: SEISMIC $(0.6-0.14SDS)D + 0.7\Omega_o E$ (Ω_o = 2.5)

Concrete Attachment: 3/4" ϕ Hilti Hit-HY 200 adhesive anchors w/ 4" embed

$Tall_{LRFD}$ = 1722 lbs $Vall_{LRFD}$ = 2032 lbs $\propto = (1 + 0.2SDS)D + 2.5E$ =1.87

$Tall_{ASD} = Tall_{LRFD}/\alpha$ = 920.9 lbs $Vall_{ASD} = Vall_{LRFD}/\alpha$ = 1086.6 lbs $(D = 0.465, E = 0.535)$

Transverse: $Uplift_{MAX}$ = 6441 lbs $Shear_{MAX}$ = 3399 lbs

$Compression_{SEISMIC}$ = 2352 lbs =[2.5*FpmaxASD*(Hcm+Hcurb)+(1+0.14S_{DS})*($WGT_{unit+curb}$/2)*wcurb]/wcurb

$Tension_{SEISMIC}$ = 1052 lbs =$Comp_{SEISMIC}$-(0.6-0.14S_{DS})*(WGTunit+curb)

$Shear_{SEISMIC}$ = 1157 lbs =2.5*FpmaxASD/2

Min Bolts Req'd Uplift = 6.99 spacing = 15.21 in o.c. Tapplied = 805.1 lbs

Min Bolts Req'd Shear = 3.13 spacing = 30.41667 in o.c. Vapplied = 242.8 lbs

Try using 8 bolts
spaced at 14.75 in o.c.

COMBINED LOADING = $\frac{T_{applied}}{T_{allow,ASD}} + \frac{V_{apllied}}{V_{allow,ASD}} \leq 1.2$ = 1.10

Use 8 - 3/4" ϕ Hilti Hit-HY 200 adhesive anchors @ 14.8 in o.c. max. along long side of curb w/ 4" embed

Longitudinal: $Uplift_{MAX}$ = 4850 lbs $Shear_{MAX}$ = 3399 lbs

$Compression_{SEISMIC}$ = 2036 lbs =[2.5*FpmaxASD*(Hcm+Hcurb)+(1+0.14S_{DS})*($WGT_{unit+curb}$/2)*Lcurb]/Lcurb

$Tension_{SEISMIC}$ = 736 lbs =$Comp_{SEISMIC}$-(0.6-0.14S_{DS})*(WGTunit+curb)

$Shear_{SEISMIC}$ = 1157 lbs =2.5*FpmaxASD/2

Min Bolts Req'd Uplift = 5.27 spacing = 12 in o.c. Tapplied = 808.3 lbs

Min Bolts Req'd Shear = 3.13 spacing = 20 in o.c. Vapplied = 242.8 lbs

Try using 6 bolts
spaced at 14.40 in o.c.

COMBINED LOADING = $\frac{T_{applied}}{T_{allow,ASD}} + \frac{V_{apllied}}{V_{allow,ASD}} \leq 1.2$ = 1.10

Use 6 - 3/4" ϕ Hilti Hit-HY 200 adhesive anchors @ 14.4 in o.c. max. along short side of curb w/ 4" embed

CURB DESIGN SUMMARY:	CBKD-92	80-265-19**	**Unit:**	Large Sunline: ZF 180
CURB RAIL THICKNESS:	0.0713 in	14 Gauge		
UNIT CLIP THICKNESS:	0.0713 in	14 Gauge		
# OF CLIPS (LONG SIDE) -	2 clips with 2 - #12 SMS screws each clip			
WEB STIFFENER:	16Ga x 3/4" x 7" (C-channel) stiffener at each clip			
# OF CLIPS (SHORT SIDE) -	2 clips with 3 - #12 SMS screws each clip			
WEB STIFFENER:	16Ga x 3/4" x 7" (C-channel) stiffener at each clip			
CORNER CONNECTION:	Use 4 - 1/4" ϕ SAE Grade 8 bolts w/ 1/4-20-UNC Threaded inserts			

CURB ANCHORAGE	WOOD	STEEL	CONCRETE
	5/8" ϕ lag screw w/ min. 3.5" embed (SGmin=0.43)	5/8" ϕ A307 bolts to steel angle below	3/4" ϕ thrd'd rod in Hilti HIT-HY 200 epoxy, min. 4" embed
LONG DIRECTION	8 @ 15.32 in o.c.	3 @ 51.63 in o.c.	8 @ 14.75 in o.c.
SHORT DIRECTION	6 @ 15.2 in o.c.	2 @ 72 in o.c.	6 @ 14.4 in o.c.